



Shorts Park, Shortstown

Flood Risk and Drainage Briefing Note

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This document has been prepared and checked in accordance with Waterman Group's IMS (BS EN ISO 9001: 2015, BS EN ISO 14001: 2015 and BS EN ISO 45001:2018)

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Cardington Retained Land
Project Number: WIE15662

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1. Introduction

- 1.1. Waterman Infrastructure and Environment have been commissioned to undertake a Flood Risk and Drainage review to support the Local Plan submission for 'Shorts Park' development located at Shortstown, Bedfordshire (hereafter referred to as 'the Site'). The Site is located in the Borough of Bedford, and within the statutory district of Bedfordshire and the River Ivel Internal Drainage Board (IDB)
- 1.2. This Briefing Note has been produced to outline the flood risk constraints relevant to the Site, and to produce a preliminary surface and foul water drainage strategy to feed into and inform the development masterplan. It is important that spatial constraints such as indicative floodplains and Sustainable Drainage Systems (SuDS) are incorporated into the scheme proposals during the early stages of the design process to ensure that the scheme is robust moving forwards.
- 1.3. The drainage strategy outlined in this note is indicative only, based on the current masterplan concept plan. As the masterplan develops, the drainage strategy would be amended accordingly.

Site Description

- 1.4. The Site is approximately 14.5 hectares (ha) in size and currently comprises predominantly grass and scrubland areas. There is also a balancing pond located in the northeast of the Site.

Figure 1: Site Location Plan



Key



Source: Google Maps

- 1.5. The topographic survey (Appendix A) indicates that ground levels across the Site fall to the east, from a high point of approximately 27.79m AOD in the western corner of the Site to a low point of

approximately 27.21m AOD adjacent to the Sites north-eastern boundary.

Proposed Development

- 1.6. The proposed development (Appendix B) would comprise a residential scheme providing up to 350 units, including a 30% provision for affordable housing, along with associated infrastructure and public open space.

2. Planning Policy

Flood Risk

- 2.1. The National Planning Policy Frameworkⁱ (NPPF) last revised in February 2019 and its supporting Planning Practice Guidanceⁱⁱ (PPG) states that inappropriate development in areas at risk of flooding should be avoided by directing development away from areas at highest risk (existing or future). Where development is necessary in such areas, the development should be made safe for its lifetime without increasing flood risk elsewhere.
- 2.2. The Bedford Borough Council (BBC) Local Flood Risk Management Strategy (LFRMS)ⁱⁱⁱ states that within the Borough, the IDBs and the Environment Agency (EA) have byelaws to protect the water corridor and manage flood risk placing restrictions on the corridor adjacent to the watercourse/river channels and flood defences that prevent them from being developed or obstructed. They also protect flood risk by placing restrictions on floodplains.
- 2.3. The watercourses within and in the vicinity of the Site area are governed by the Bedfordshire and River Ivel IDB. The IDB have stated that any watercourse or land drainage ditch within the Board's area is subject to its byelaws, the most pertinent being:
 - No development will be permitted within 9m of a watercourse, measured from the top of bank on both sides of the watercourse

Surface Water Drainage

- 2.4. The NPPF states that major developments should incorporate SuDS unless there is clear evidence that this would be inappropriate. The systems used should:
 - Take account of advice from the Lead Local Flood Authority (LLFA) and IDB where applicable;
 - Have appropriate proposed minimum operational standards;
 - Have maintenance arrangements in place to ensure an acceptable standard of operation for the lifetime of the development, and
 - Where possible, provide multifunctional benefits.
- 2.5. The BBC LFRMS states that in their aim to provide resilience against climate change, SuDS should be delivered through the planning process, with all major developments managing rainwater and surface water that replicated natural drainage.
- 2.6. The BBC Supplementary Planning Document (SPD) for SuDS^{iv} states that the most preferable options for drainage are discharge into ground (minimum acceptable rate of 1×10^{-5} m/s) and discharge into a surface water body, with the least preferential option being discharge into a surface water sewer.
- 2.7. BBC as LLFA have the following requirements relating to surface water drainage:
 - Attenuation should be designed to accommodate flows for the 1 in 100 year event plus a 40% allowance for climate change;
 - 10% allowance for urban creep; and
 - Discharge rate as agreed by Bedford and River Ivel IDB.
- 2.8. The Bedfordshire and River Ivel IDB also require that any surface water discharge should be restricted to 4 l/s per contributing impermeable hectare.

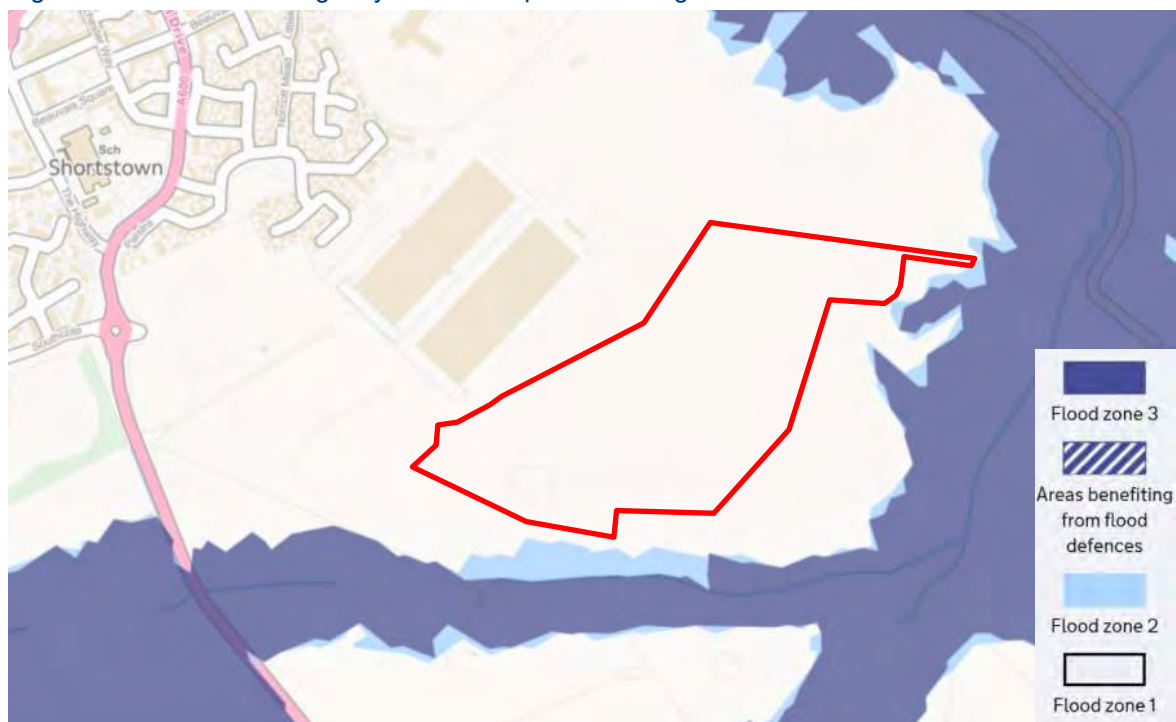
- 2.9. For discharge into the ground, the Bedford Borough Council SPD limits the acceptable depth of infiltrating SuDS to 2.0m below ground level (bgl), with a minimum of 1.2m clearance between the base of the infiltration SuDS and peak seasonal groundwater levels. It further states that at steep sites with permeable superficial deposits and impermeable bedrock, infiltrating SuDS could result in sloping instabilities, requiring a geotechnical investigation to confirm the feasibility.
- 2.10. Appropriate pollution prevention measures should be incorporated into the design. As per the Bedford Borough Council SPD, clean water from roofs can be directly discharged to any soakaway or watercourse. The SuDS Mitigation Index approach as per the CIRIA SuDS Manual^v should be used for runoff from all other hardstanding areas.

3. Flood Risk


Tidal/Fluvial

- 3.1. There are no tidal waterbodies within the vicinity of the Site, therefore the risk of flooding to the Site from tidal sources is negligible.
- 3.2. The EA's Flood Map for Planning (Figure 2) shows that the Site is located within Flood Zone 1, land defined as having less than 0.1% annual probability (1 in 1000 year) of flooding and is therefore considered to be at low risk of flooding.

Figure 2: Environment Agency's Flood Map for Planning



Key

-  Site Boundary

Source: <https://flood-map-for-planning.service.gov.uk>

- 3.3. According to the Bedford Borough Council LFRMS, the primary source of flood risk throughout the Borough is fluvial from the River Great Ouse, which is classified as a Main River. The River Great Ouse is located approximately 1.7km to the north of the Site. The Cople Brook flows to the east and is located a maximum of 100m to the south of the Site.
- 3.4. As part of a recent modelling study carried out by Waterman, a new fully linked 1D/2D hydraulic model was built to improve the representation of flood risk from the Cople Brook. The results of this modelling study are included in Appendix C. The results indicate that the Site would remain free of flooding during all events up to and including the present day 1 in 1000 year, and the future 1 in 100 year events for the year 2100, taking account of the impacts of climate change.
- 3.5. As such, it is considered that the risk of fluvial flooding to the Site is low.

Pluvial

- 3.6. Pluvial flooding, also known as surface water flooding, can occur when natural and engineered drainage systems have insufficient capacity and are overwhelmed by the volume of rainfall. Pluvial flooding can occur in rural areas during medium intensity, long duration rainfall events where saturated ground conditions prevent infiltration into the subsoil. This flood water would then be conveyed via overland flow routes dictated by the local topography.

Overland Surface Water

- 3.7. The EA's Flood Risk from Surface Water mapping sets out a high level, national scale assessment of pluvial flooding at the Site. The mapping shows (Figure 3) that the Site is predominantly at 'very low risk' of flooding (less than 0.1% annual probability), however, some areas are indicated to be at 'low risk' (between 0.1% and 1% annual probability) to 'high risk' (greater than 3.3% annual probability).

Figure 3: Environment Agency's Flood Risk from Surface Water Mapping



Key

-  Site Boundary

Source: <https://flood-warning-information.service.gov.uk>

- 3.8. During the 'high risk' event, the areas adjacent to the north-western side of Hangar 1 are predicted to flood to depths of below 0.3m. However, the remainder of the Site would be free of flooding.
- 3.9. During the 'medium risk' event (between 1% and 3.3% annual probability), flooding adjacent to Hangar 1 would be more extensive and could potentially reach depths of up to 0.9m in a small, isolated area to the northwest of the Hangar. A few small, isolated areas to the west of the Hangar would also become at risk of flooding to depths of up to 0.3m during the 'medium risk' scenario.
- 3.10. During the 'low risk' event, a larger area to the northwest of the Hangar would become at risk of

flooding to depths of up to 0.9m, and a larger area to the west of the Hangar would become at risk of flooding to depths of below 0.3m. In addition to the increases in flood extent in these areas, several areas to the centre and east of the Site would become at risk of flooding. The flood depths in these areas would be predominantly below 0.3m, however, a small area to the south of Hangar 2 could potentially flood to depths of up to 0.9m.

- 3.11. Upon reviewing this mapping against the topographic survey for the Site, it is noted that the areas indicated to be at risk of flooding during the 'medium risk' and 'low risk' events are topographically low spots and it is likely that any rainfall on these areas would become trapped and be unable to drain away. The only area that could potentially be part of an overland flow route would be the area of flooding to the southwest of Hangar 1 and 2, and this area is not indicated to flood to a significant extent.
- 3.12. There is an existing ditch along to the western boundary of the Site, adjacent to the A600. However, the mapping indicates that this ditch does not pose a risk of flooding to the Site.
- 3.13. The areas of the Site where residential accommodation is proposed are shown to be predominantly at 'very low risk' of flooding, with a small area shown to be at 'low risk'. The proposed surface water drainage strategy would manage any rainwater falling on these areas of the Site.
- 3.14. It is therefore considered that the risk of pluvial flooding to the Site is predominantly 'very low risk', with some areas being at 'low risk' and 'medium risk'.

Sewers

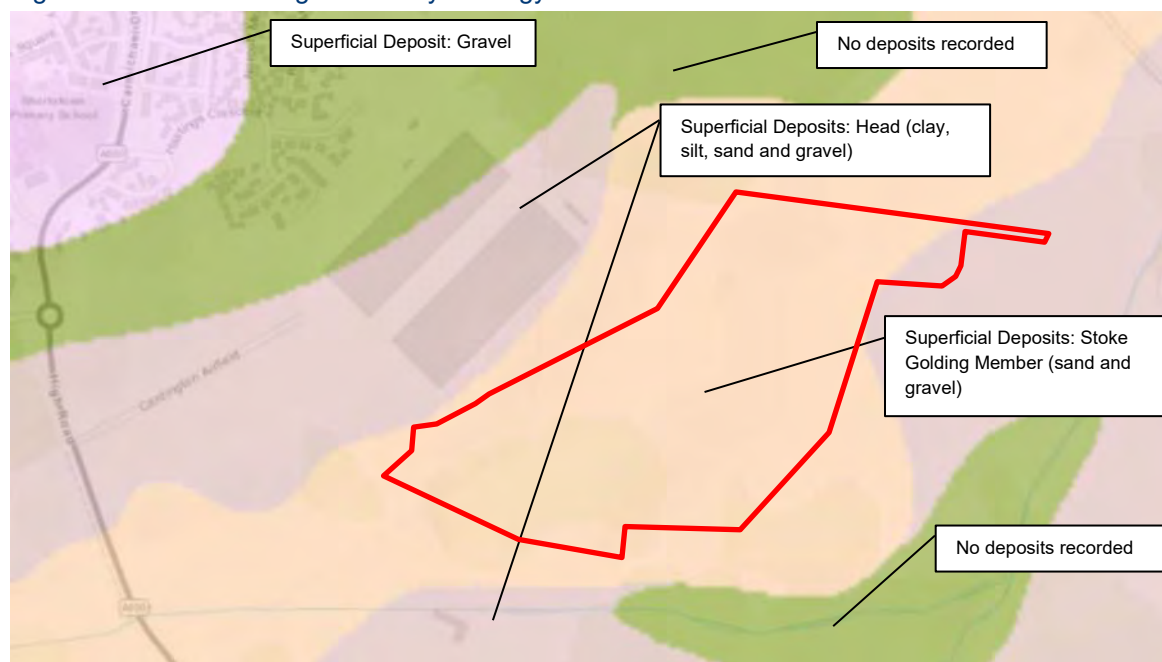
- 3.15. The BBC LFRMS states that 5,280 properties have been identified as being at risk of surface water flooding in the Borough, with 3,400 of these located within Bedford town. It states that this surface water flooding relates to urban areas where rapid runoff from impermeable areas exceeds the drainage capacity, i.e. sewer flooding.
- 3.16. As the Site is currently comprised of mostly undeveloped land, the risk of flooding due to insufficient capacity and potential blockages in the local drainage system is considered negligible. Therefore, the risk of sewer flooding to the Site is considered to be low.

Groundwater

- 3.17. Groundwater flooding is most likely to occur in low-lying areas underlain by permeable sub-surface geology, where groundwater emerges due to a high water table following prolonged periods of heavy rainfall.
- 3.18. The natural geological sequence beneath the Site has been established through a review of the British Geological Survey (BGS) online Geology of Britain Viewer (Figure 4, overleaf) and historic borehole records. According to the BGS mapping, the Site is underlain by the Stoke Golding Member in the southern areas, comprised of sand and gravel. In the northern areas of the Site, the underlying geology is Head deposits, comprised of clay, silt, sand and gravel, with some areas to the north of Hangar 1 with no superficial deposits recorded. The bedrock geology at the Site is the Peterborough Mudstone Member.
- 3.19. Several boreholes located to the southwest of Hangar 2, directly adjacent to the Sites boundary, were reviewed and it was found that groundwater was encountered at depths of approximately 2.5m to 3.0m below ground level (bgl).
- 3.20. According to the BBC LFRMS, none of the underlying bedrock in the Borough support major aquifers,

indicating that they are unlikely to have the potential to store and transmit large quantities of water. Consequently, the LFRMS states that the risk of groundwater flooding in the borough is generally considered to be low. However, where there are superficial deposits of sand and gravel, such as those present on-site, there is the potential for groundwater to be present, as confirmed through the historic borehole records.

Figure 4: British Geological Survey Geology of Britain Viewer



Key

 Site Boundary

Source: <http://mapapps.bgs.ac.uk/geologyofbritain3d/>

- 3.21. The LFRMS confirms that the EA, the IDB and BBC hold no records of historic groundwater flooding in the area.
- 3.22. It is therefore considered that while the risk of groundwater flooding to the Site is low, if basement development is proposed, this would need to demonstrate that it would not significantly displace any potential groundwater flows beneath the Site which could increase flood risk off-site. Also, any proposed basements would need to be adequately waterproofed to prevent the ingress of groundwater.

Artificial Sources

- 3.23. The EA's Flood Risk from Reservoirs mapping presents the largest extent that could potentially be affected in the event of a reservoir failure. The mapping shows (Figure 5, overleaf) that the Site would not be affected in such a flood event.
- 3.24. The development proposals (Appendix B) show that there are two large attenuation ponds located to the northeast of the Site and to the west of the Site, which form part of the surface water drainage strategy for the wider Cardington development. The ponds are therefore likely to be managed and maintained appropriately and the risk of overtopping from these ponds is considered to be low.

3.25. The risk of flooding from artificial sources is therefore considered to be low.

Figure 5: Environment Agency's Flood Risk from Reservoirs



Key

 Site Boundary

Source: <https://flood-warning-information.service.gov.uk>

4. Surface Water Drainage

- 4.1. A preliminary surface water drainage strategy is outlined below, in order to ensure that the considerations and spatial requirements of SuDS features are incorporated into the early stages of design.

Existing Drainage

- 4.2. Anglian Water sewer records (Appendix D) indicate a number of public sewers located in the vicinity of the Site. These are summarised in Table 1 below.

Table 1: Existing Sewers

Location	Sewer
Greycote Roundabout (west of the Site)	Anglian Water surface water sewer with outfall to assumed ditch (600mm diameter)
A600 (west of the Site)	Anglian Water foul water rising main (100mm diameter)

- 4.3. As the Site is predominantly undeveloped land, it is anticipated that the existing drainage regime is a combination of natural shallow infiltration and overland flows towards the Cople Brook, as dictated by the Site's topography.

Discharge Rate and Location

- 4.4. The Bedfordshire and River Ivel IDB has confirmed that they require surface water discharge into watercourse under their management to be restricted to 4 l/s/ha applied to the proposed impermeable area.
- 4.5. Due to the Site being mostly undeveloped (i.e. greenfield), attenuation should be provided in the most sustainable manner through the use of SuDS. Water quality should be considered to ensure that any potential pollutants are managed appropriately prior to being discharged.
- 4.6. The Building Regulations and Planning Policy Guidance set out a hierarchy of surface water discharge, which should be adhered to in decreasing order of preference:
- I. Discharge to ground;
 - II. Discharge to a surface water body;
 - III. Discharge to a surface water sewer; and
 - IV. Discharge to a combined sewer.
- 4.7. BGS records indicate that the Site is underlain by the generally impermeable Peterborough Mudstone Member. However, a significant portion of the Site is underlain by superficial deposits of the Stoke Golding Member, comprised of sands and gravel. Therefore, drainage via infiltration may be possible in these areas. It is recommended that soakage testing is undertaken as soon as possible to determine the infiltration rates at the Site and the feasibility of discharge to ground. The Site is not located within a Source Protection Zone as defined by the EA and it is not anticipated that there would be contamination within the ground as it is currently predominantly undeveloped.
- 4.8. The Cople Brook is located a short distance to the south of the Site and is located within the client's

ownership boundary. In lieu of soakage testing results and in line with the drainage hierarchy, it is proposed to discharge surface water runoff into this Brook.

Surface Water Drainage Strategy

- 4.9. To reduce the need for surface water pumping from lower areas, the proposed drainage strategy assumes that all surface water runoff will drain to a single attenuation feature located centrally along the Sites southern boundary. This strategy aims to keep the length of drainage runs as short as possible. Attenuation would be provided within a detention basin (Appendix E) which would then discharge to the Cople Brook located to the south of the Site.
- 4.10. The Bedfordshire and River Ivel IDB require that any surface water discharge should be restricted to 4 l/s per contributing impermeable hectare. Based on a proposed residential area of 8.11ha (Appendix B), an assumed 70% proposed impermeable area has been used for the calculations and including a 10% allowance for urban creep, the total impermeable area for the calculations would be 6.24ha (77% of the residential development area of 8.11ha). This results in a discharge rate of 25.0 l/s for the Site.
- 4.11. MicroDrainage Source Control module has been used to calculate the required attenuation volumes for the 1 in 100 year event plus a 40% allowance for climate change (Appendix F), in order to restrict runoff from the Site to 4 l/s/ha. Source Control considers all storm durations and return periods. Based on a restriction to 25.0 l/s, approximately 4955m³ of attenuation would be required at the Site (Appendix F).
- 4.12. The Site has very shallow topography, indicating that a gravity discharge may be difficult to achieve, even with shallow pipe gradients. To reduce the depths required within piped drainage runs, a 1.5m deep conveyance swale is proposed along the south-western boundary of the Site, and a 1m deep swale is proposed along the eastern boundary of the Site (Appendix E). This provides a deep, open conveyance feature which piped drainage runs from the Sites extremities can connect into, thereby minimising the amount of ground raising required.
- 4.13. Based on a strategy including the above swales, approximately 0.2m of ground raising would be required in the southwest of the Site, rising to approximately 1.5m in the north-western corner (Appendix E). Approximately 0.7m of ground raising would be required in the northeast of the Site in order to achieve a minimum gradient for the conveyance swale along the eastern boundary (Appendix E). It is anticipated that the cut obtained from excavating the detention basin could be repurposed to achieve some of the fill requirements.
- 4.14. The amount of ground raising required would be subject to the developing masterplan and would be confirmed during detailed design with cut and fill calculations. Alternatively, shallow swales could be incorporated within the verges of proposed roads to provide a high-level conveyance route for runoff to avoid ground raising.
- 4.15. The strategy shown is very tight in terms of levels and further design development might result in more ground raising being required to achieve gravity discharge than would be considered cost effective and sustainable. In this event, surface water would need to be pumped to the basins, however, this is considered to be a less sustainable and therefore, less favourable option.
- 4.16. The following assumptions have been made for the purposes of this high-level assessment:
 - Percentage impermeable area (PIMP): 70% including a 10% allowance for urban creep (77% total PIMP);
 - Detention basins designed as dry, with no permanent water level;

- Detention basin dimensions: 2m depth, 1 in 4 side slopes, 9m maintenance buffer;
- 1 in 200 minimum pipe gradients, to achieve self-cleaning velocities;
- 1 in 500 minimum gradient for the swale, to maintain a positive flow direction; and
- The areas included within the drainage catchments only include the area to the south and east of Hangar 2, in line with the Concept Plan (Appendix B).

Water Quality

- 4.17. The most sustainable way to drain surface water is through the use of SuDS, which need to be considered in relation to site-specific constraints. SuDS mimic the natural drainage system and provide a method of surface water drainage which can decrease the quantity of water discharged, and hence reduce the risk of flooding. In addition, to reducing flood risk, SuDS features improve water quality and provide biodiversity and amenity benefits.
- 4.18. At this stage of the scheme design, the basins were sized in order to contain all of the required storage volume for each catchment. However, in order to fulfil the treatment requirements outlined in the CIRIA SuDS Manual, and to maximise the benefits provided, it is recommended that a range of SuDS features are considered. This is outlined in Table 3 below.

Table 2: Sustainable Drainage Techniques

Device	Description	Comments	✓/✗
Green / brown roofs (source control).	Provide soft landscaping at roof level which reduces surface water runoff.	Depending on the pitch of proposed roofs, green/brown roofs could potentially be incorporated. This would be dependent on the proposed use of the building as these features are generally not considered suitable for private residential dwellings. Therefore, appropriate locations may be limited	✓
Pervious surfaces (source control).	Storm water is allowed to infiltrate through the surface into a storage layer, from which it can either infiltrate and / or slowly release to sewers.	Pervious surfacing could potentially be included within the proposed private car parking areas, and areas not likely to be trafficked by HGVs. Infiltration may be feasible across a large part of the Site, subject to soakage, therefore, there is the potential for unlined pervious surfaces which allow infiltration through the sub-base	✓
Rainwater harvesting (source control).	Reduces the annual average rate of runoff from a site by reusing water for non-potable uses e.g. toilet flushing or water butts.	There are no constraints to the incorporation of rainwater harvesting. However, the reduction in surface water runoff from the Site cannot be quantified with certainty as this would be dependent on the demand for harvested rainwater. Water butts could be considered for individual properties	✓
Swales (permeable conveyance).	Broad shallow channels that convey / store runoff, and allow infiltration (ground conditions permitting).	Deep swales are recommended to the west and east of the Site to provide an open conveyance feature in order to minimise the amount of land raising required. Swales could also be designed to allow for infiltration, subject to soakage testing. The attenuation provided in swales would be quantified as the masterplan progresses into more detail	✓

Device	Description	Comments	✓/✗
Filter drains & perforated pipes (permeable conveyance).	Trenches filled with granular materials (which are designed to take flows from adjacent impermeable areas) that convey runoff while allowing infiltration (ground conditions permitting).	Infiltration may be feasible for large parts of the Site, subject to soakage testing. Filter drains could be incorporated into the development to provide treatment as well as shallow conveyance.	✓
Bioretention Systems / Rain Garden (end of pipe treatment).	A shallow landscaped depression which allows runoff to pond temporarily on the surface before filtering through vegetation and underlying soils.	The incorporation of bioretention systems/rain gardens is encouraged as they would provide amenity benefits as well as attenuation and treatment of runoff. These features could also be designed to allow for infiltration, dependent on soakage testing.	✓
Ponds (end of pipe treatment)	Depressions in the surface designed to store runoff without infiltration through the base.	Dry ponds are recommended as the primary form of SuDS for the development. Ponds would provide amenity and biodiversity benefits as well as the main function of providing attenuation.	✓

5. Foul Drainage

- 5.1. The proposed foul drainage system would be designed in accordance with BS EN 752 – Drain and Sewer Systems Outside Buildings, BS EN 12056 – Gravity Drainage Systems Inside Buildings and Approved Document H of Building Regulations.
- 5.2. As the Site is currently predominantly greenfield land, there are no public foul sewers within the Site. Based on Anglian Water asset records (Appendix D), there is a foul water rising main located within the A600 to the west of the Site, which rises from a pumping station located approximately 500m to the south of the Site, in the vicinity of Gorse Farm. Connections into existing rising mains are typically considered unacceptable, therefore an alternative solution would be required. The asset plans also indicate an existing foul water pumping station to the north of Hangar 1 (Appendix D).
- 5.3. Due to the topography of the Site, pumping of foul water would be required for the entire Site. An estimated 2 no. pumping stations would be required at the Site with associated rising mains, shown indicatively on the Preliminary Drainage Strategy Plan (Appendix E).
- 5.4. Off-site sewer connections would need to be requisitioned from Anglian Water, under S.98 of the Water Industry Act 1991, to serve the Site. Towards the southern edge of the Site, 1 no. pumping station would be required to serve the southern and western parts of the Site. To the north-eastern end of the Site, 1 no. pumping station would be required to serve the northern and eastern parts of the Site. Due to the uncertainty surrounding the capacity of the existing foul water sewer and pumping station to the north of Hangar 1, it is likely that the Site would discharge into the existing foul sewer network located within Southcote Road.
- 5.5. Due to the increase in foul flows post-development, it is anticipated that the existing Anglian Water pumping station would require some upgrades to accommodate the additional flows. The proposed foul flows would be quantified as the masterplan develops, and a pre-development enquiry would be submitted to Anglian Water to confirm the preferred discharge points to the public sewer network and the level of upgrades required.

6. Conclusions and Recommendations

- 6.1. The Site is located within Flood Zone 1, denoting a low annual probability of flooding from fluvial sources.
- 6.2. The risk of pluvial flooding to the Site has been confirmed as 'very low risk', with some areas being at 'low risk' and 'medium risk'. The risk of overland surface water flooding would be managed appropriately within the proposed drainage strategy.
- 6.3. The risk of flooding to the Site from groundwater and artificial sources has also been assessed and found to be low.
- 6.4. The Cople Brook, to the south and east of the Site is classified as an Ordinary Watercourse and falls under the responsibility of the Bedfordshire and River Ivel IDB. No development or ground raising would be acceptable within 9m of this watercourse.
- 6.5. The underlying geology at the Site (superficial deposits of gravel and sand) suggest that infiltration may be feasible for large parts of the Site. It is recommended that soakage testing is undertaken to confirm the infiltration rates at the Site.
- 6.6. In lieu of infiltration rates, surface water runoff from the Site would be restricted to 4 l/s/ha as requested by the IDB and attenuated within a detention basin prior to discharging to the Cople Brook. Based on a restriction to this rate, approximately 4955m³ of attenuation would be required at the Site. As the masterplan proposals are developed, the drainage strategy would need to be updated to suit.
- 6.7. The Site has very shallow topography, therefore ground raising would be required within the western, northern and eastern corners of the Site to achieve a gravity discharge to the detention basin. To minimise the amount of ground raising required, deep swales have been incorporated along the south-western and eastern boundaries to convey surface water runoff from the extremities of the Site.
- 6.8. The possibility of including a wide variety of SuDS should be assessed as the masterplan progresses, including pervious paving, green/brown roofs, rain gardens and rainwater harvesting.
- 6.9. Foul flows from the development would likely be discharged via pumped rising mains to the nearby public sewer system within Southcote Road. Off-site sewer connections would need to be requisitioned from Anglian Water to serve the Site.
- 6.10. Due to the increase in foul flows post-development, it is anticipated that upgrades would be required to the Anglian Water sewer network to accommodate the additional flows. The proposed foul flows would be quantified as the masterplan develops, and a pre-development enquiry would be submitted to Anglian Water to confirm the preferred discharge points and the level of upgrades required.
- 6.11. The proposed masterplan has been designed in accordance with the recommendations included within this report and is therefore considered suitable for development from a flood risk and drainage perspective.

7. References

ⁱ Ministry of Housing, Communities and Local Government, last revised February 2019. National Planning Policy Framework.

ⁱⁱ Ministry of Housing, Communities and Local Government, March 2014. Planning Practice Guidance.

ⁱⁱⁱ Bedford Borough Council, November 2015. Local Flood Risk Management Strategy.

^{iv} RAB Consultants/Bedford Borough Council, February 2018. Supplementary Planning Document for Sustainable Drainage Systems.

^v CIRIA C753, 2015. The SuDS Manual.



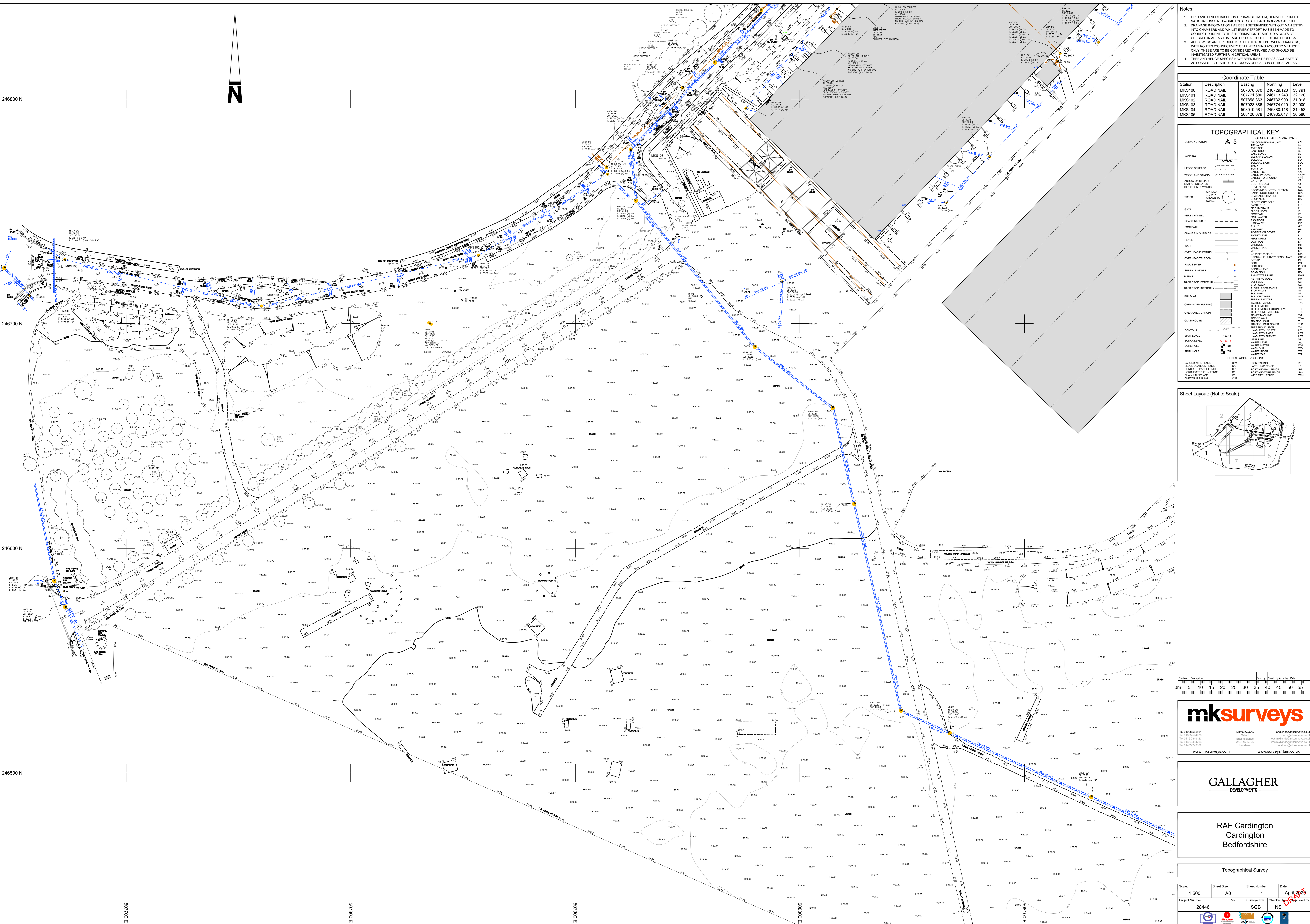
APPENDICES

A. Topographic Survey

Appendices

Cardington Retained Land
Project Number: WIE15662

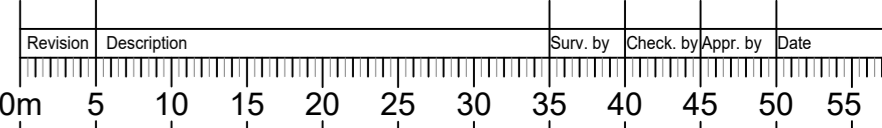
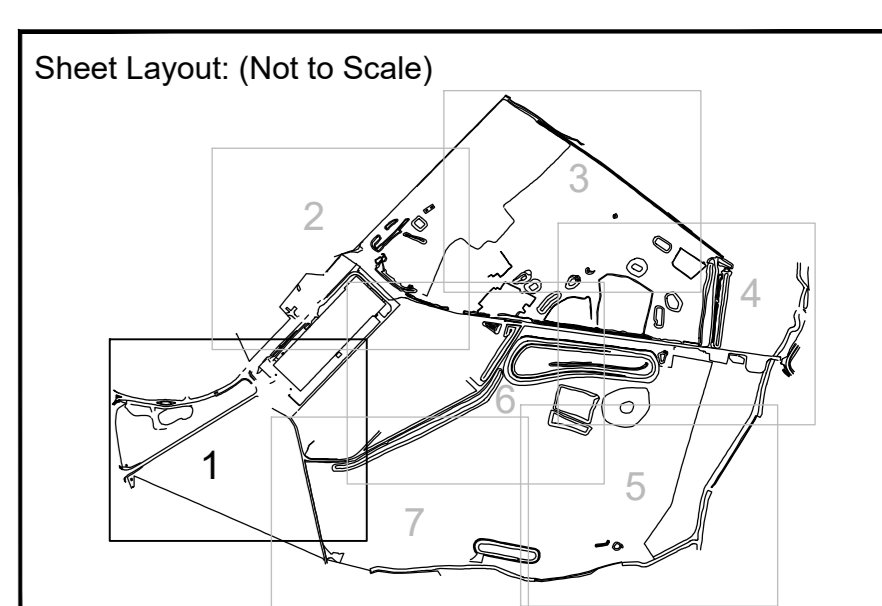
Document Reference: WIE15662-109-BN-5-4-1-Flood



- Notes:
- GRID AND LEVELS BASED ON ORDNANCE DATUM, DERIVED FROM THE NATIONAL GRID NETWORK. LOCAL SCALE FACTOR 0.99996 APPLIED.
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Coordinate Table				
Station	Description	Easting	Northing	Level
MKS100	ROAD NAIL	507678.670	246729.123	33.791
MKS101	ROAD NAIL	507771.680	246713.243	32.120
MKS102	ROAD NAIL	507858.383	246732.999	31.918
MKS103	ROAD NAIL	507928.386	246774.010	32.000
MKS104	ROAD NAIL	508019.581	246880.118	31.453
MKS105	ROAD NAIL	508120.678	246985.017	30.588

TOPOGRAPHICAL KEY		
SURVEY STATION		5
BANKING		TOP BOTTOM
HEDGE SPREADS		
WOODLAND CANOPY		
ARROWS ON STEPS		
PUMP INDICATORS		
TREES		
GATE		
SEWER CHANNEL		
ROAD UNNUMBERED		
FOOTPATH		
CHANGE IN SURFACE		
FENCE		
WALL		
OVERHEAD ELECTRIC		
FOCUS SINKER		
SURFACE SEWER		
PUMP		
BACK DROP (EXTERNAL)		
OPEN SIDED BUILDING		
CLASSIFIED		
CONTOUR		± 0.15 (0.2) ± 0.25 (0.3)
BORER HOLE		
TRIAL HOLE		



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 Tel 01608 665677 | Email: enquiries@mk-surveys.co.uk
 Tel 01608 665622 | Email: enquiries@mk-surveys.co.uk
 Tel 01608 241992 | Email: enquiries@mk-surveys.co.uk

www.mk-surveys.com | www.surveys4bm.co.uk

GALLAGHER DEVELOPMENTS

RAF Cardington
Cardington
Bedfordshire

Topographical Survey

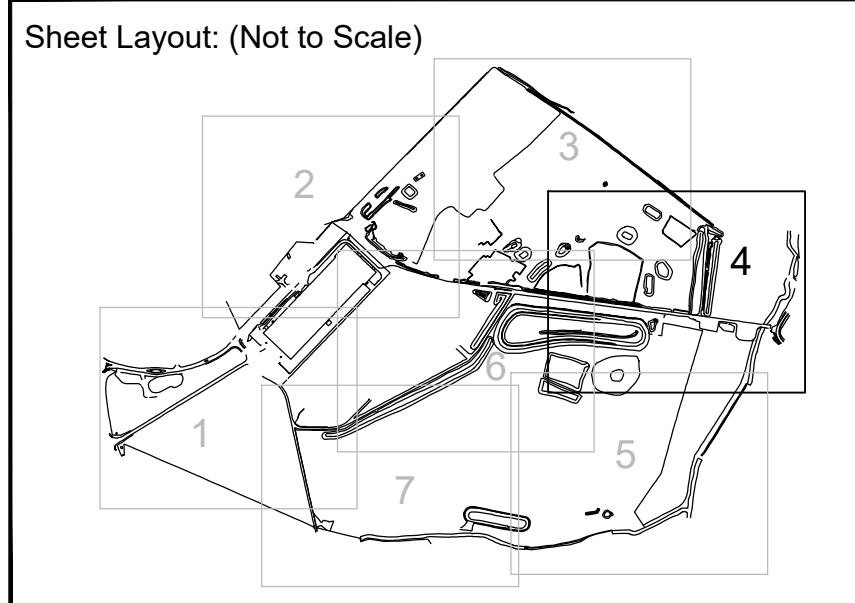
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Project Number	28446	Rev	NS	Surveyed by	NS	Checked by	NS

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MKS103	ROAD NAIL	507928.396	246774.010	32.000
MKS104	ROAD NAIL	508019.581	246880.118	31.453
MKS105	ROAD NAIL	508120.678	246985.017	30.588

TOPOGRAPHICAL KEY

<p>5 SURVEY STATION</p> <p>BANKING</p> <p>HEDGE SPREADS</p> <p>WOODLAND CANOPY</p> <p>ARROW ON STEPS</p> <p>DIRECTION UNPARALLEL</p> <p>TREES</p> <p>GATE</p> <p>KERB CHANNEL</p> <p>ROAD UNWEATHERED</p> <p>FOOTPATH</p> <p>CHANGE IN SURFACE</p> <p>FENCE</p> <p>WALL</p> <p>OVERHEAD ELECTRIC</p> <p>RAIL BEWER</p> <p>SURFACE SEWER</p> <p>P/FRAP</p> <p>BACK DROP (EXTERNAL)</p> <p>BUILDING</p> <p>OPEN SIDED BUILDING</p> <p>OVERWANG / CANOPY</p> <p>CLASSHOUSE</p> <p>CONTOUR</p> <p>SPOT LEVEL</p> <p>SCAMER LEVEL</p> <p>BORE HOLE</p> <p>TRIAL HOLE</p>	<p>5 GENERAL ABBREVIATIONS</p> <p>5 AIR VALVE</p> <p>5 BACK DROP</p> <p>5 BENCH MARK</p> <p>5 BOLLARD</p> <p>5 BOLLARD LIGHT</p> <p>5 BRICK</p> <p>5 CABLE TRAY</p> <p>5 CABLE TV COVER</p> <p>5 CONTROL BOX</p> <p>5 COVER LEVEL</p> <p>5 COUNTER CONTROL BUTTON</p> <p>5 DRAINAGE CHAMBER</p> <p>5 DRAINAGE CHAMBER</p> <p>5 ELECTRICITY POLE</p> <p>5 GATE</p> <p>5 FIRE HYDRANT</p> <p>5 FLOOR LEVEL</p> <p>5 FUEL WATER</p> <p>5 GAS VALVE</p> <p>5 GRATE</p> <p>5 HARD BED</p> <p>5 MANHOLE</p> <p>5 MANHOLE POST</p> <p>5 MET</p> <p>5 METERS VISIBLE</p> <p>5 P/FRAP</p> <p>5 POST BOX</p> <p>5 PRODUCE PIPE</p> <p>5 ROAD SIGN</p> <p>5 RAIN WATER PIPE</p> <p>5 RAMP</p> <p>5 SOFT BED</p> <p>5 STOP DOCK</p> <p>5 STREET NAME PLATE</p> <p>5 STOP VALVE</p> <p>5 SURFACE WATER</p> <p>5 TACTILE PAVING</p> <p>5 TELECOM INSPECTION COVER</p> <p>5 TELEPHONE CALL BOX</p> <p>5 TOWER</p> <p>5 TRAFFIC LIGHT COVER</p> <p>5 THREE PEG LEVEL</p> <p>5 UNABLE TO LOCATE</p> <p>5 UTILITY</p> <p>5 VENT PIPE</p> <p>5 WATER LEVEL</p> <p>5 WATER METER</p> <p>5 WINDY OUT</p> <p>5 WATER RISER</p> <p>5 WATER TAP</p>	<p>5 FENCE ABBREVIATIONS</p> <p>5 BARBED WIRE FENCE</p> <p>5 CLOSE BOARDED FENCE</p> <p>5 CONCRETE PANEL FENCE</p> <p>5 CORRUGATED IRON FENCE</p> <p>5 CHAIN LINK FENCE</p> <p>5 CHESTNUT PALING</p> <p>5 B&W</p> <p>5 LARCH LAP FENCE</p> <p>5 POST AND RAIL FENCE</p> <p>5 POST AND WIRE FENCE</p> <p>5 WIRE MESH FENCE</p> <p>5 WRM</p>
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Scale: 1:500

Project Number: 28446

Scale Bar: 0m to 55m

Revision Table:

Rev	Description	By	Date
1	Issue for Tender	NS	2024

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28446

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GALLAGHER DEVELOPMENTS

RAF Cardington
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Bedfordshire

Topographical Survey

Scale: 1:500

Sheet Size: A0

Sheet Number: 4

Date: April 2024

Project Number: 28446

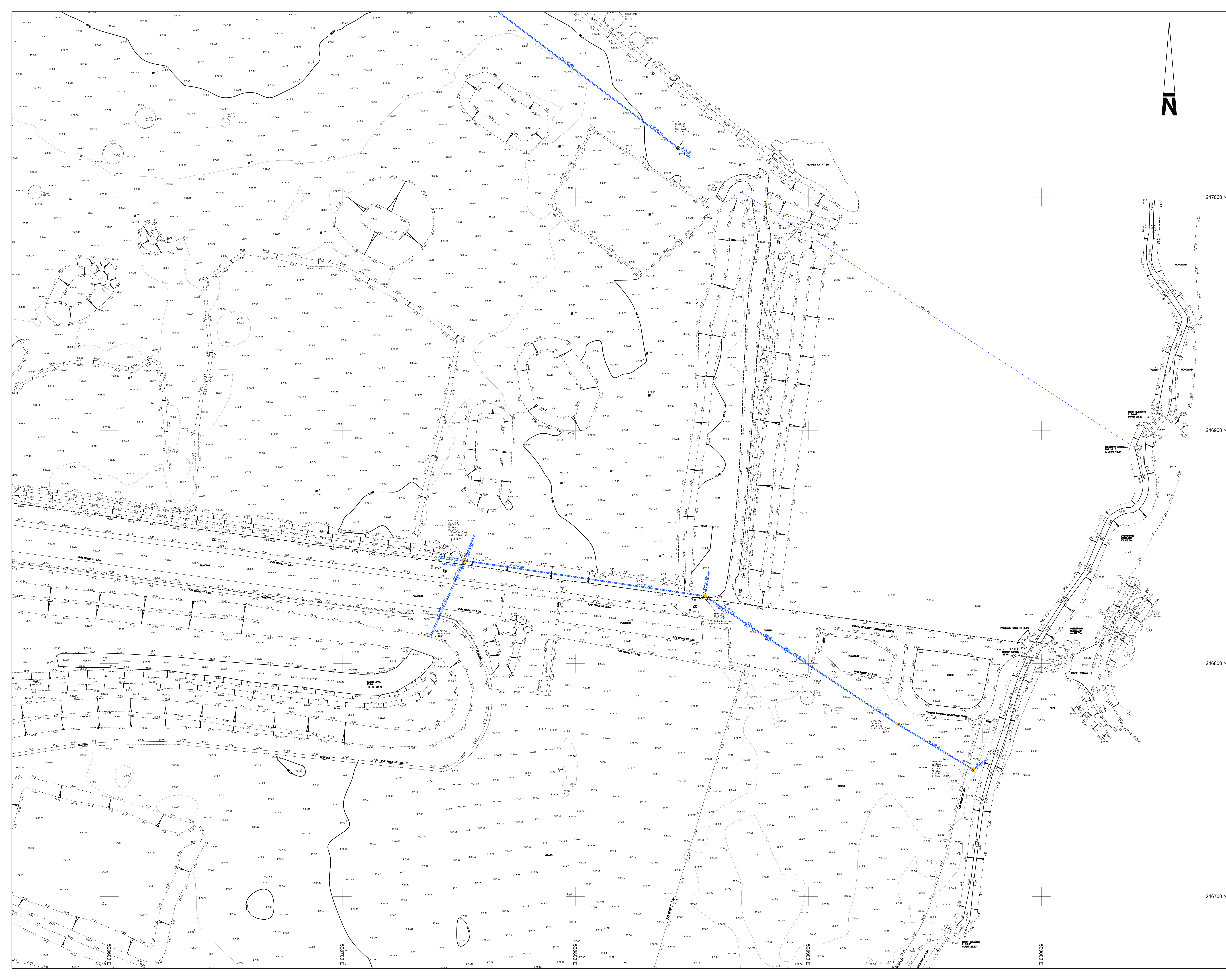
Rev: -

Survised by: SGB

Checked by: NS

Drawn by: -

Approved by: -

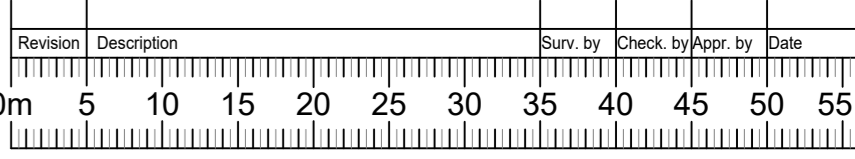
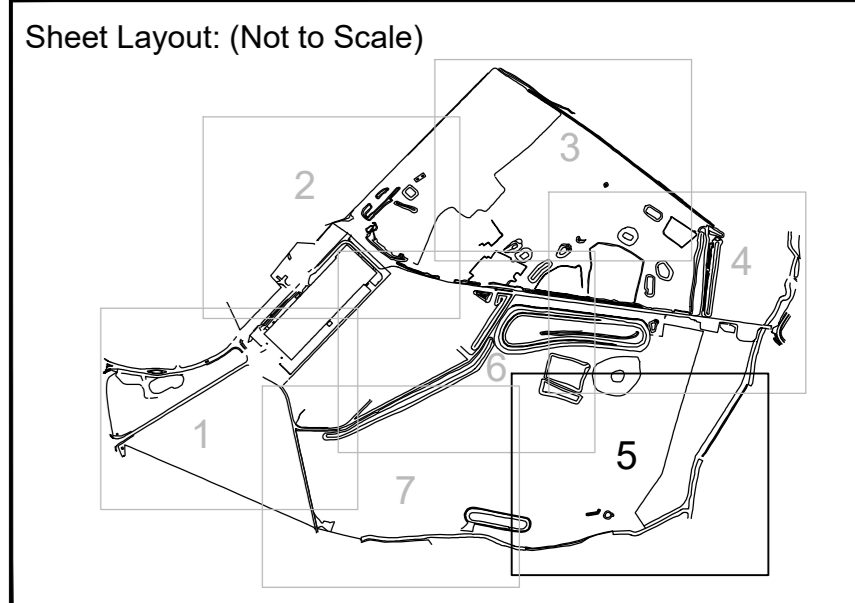


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TOPOGRAPHICAL KEY

	5	GENERAL ABBREVIATIONS	ACU	ACU
		ARE COVERING UNIT	AV	AV
		BACK DITCH	BD	BD
		BASE LEVEL	BL	BL
		BOLLARD	BO	BO
		BOLLARD LIGHT	BLD	BLD
		BRICK	BR	BR
		BRICK PAVED	BRP	BRP
		BRICK TYPED	BRTP	BRTP
		BRICK TYPED	BRTP	BRTP
		BRICK TYPED	BRTP	BRTP
		BRICK TYPED	BRTP	BRTP
		BRICK TYPED	BRTP	BRTP
		BRICK TYPED	BRTP	BRTP
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		BRICK TYPED	BRTP	BRTP



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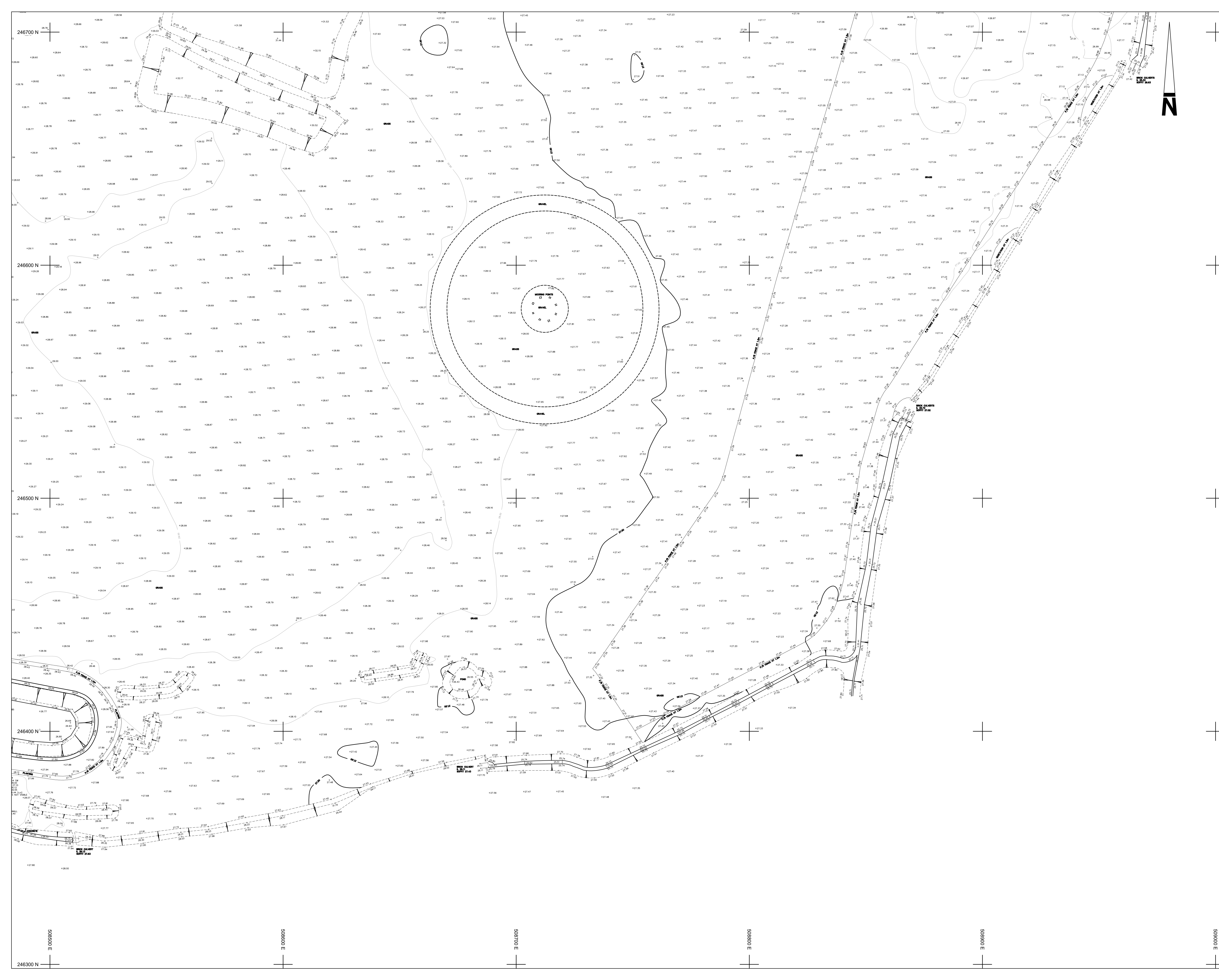
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 info@mk-surveys.co.uk
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GALLAGHER DEVELOPMENTS

RAF Cardington
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Topographical Survey

Scale: 1:500	Sheet Size: A0	Sheet Number: 5	Date: April 2024
Project Number: 28446	Rev: -	Surveyed by: SGB	Checked by: SGB

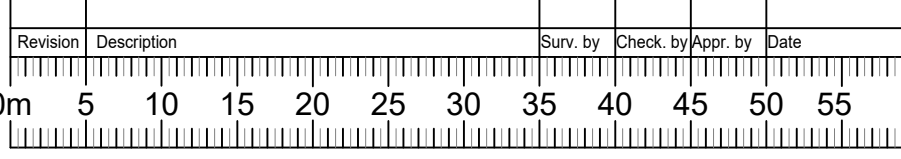
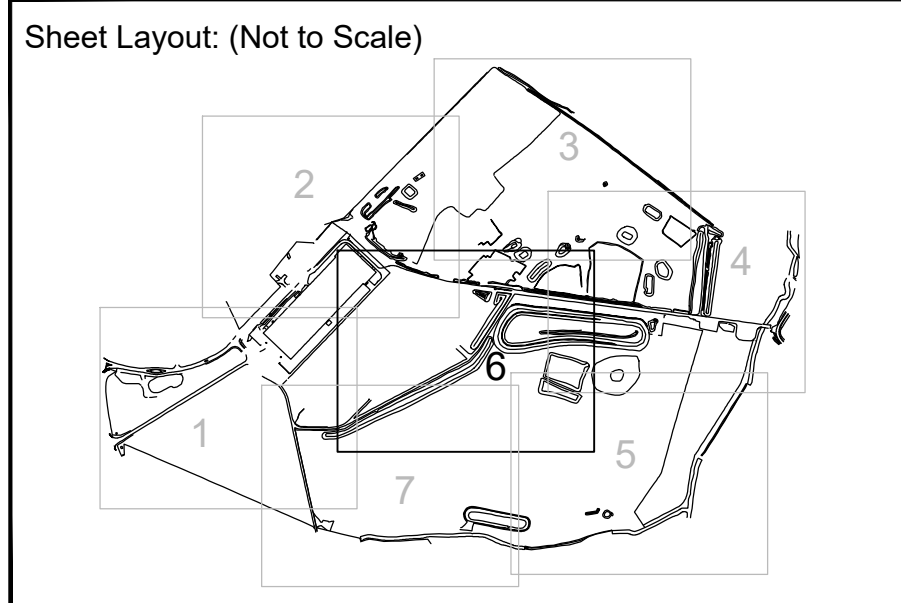


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<p>CV</p> <p>CW</p> <p>CX</p> <p>CY</p> <p>CZ</p> <p>DA</p> <p>DB</p> <p>DC</p> <p>DD</p> <p>DE</p> <p>DF</p> <p>DG</p> <p>DH</p> <p>DI</p> <p>DJ</p> <p>DK</p> <p>DL</p> <p>DM</p> <p>DN</p> <p>DO</p> <p>DP</p> <p>DQ</p> <p>DR</p> <p>DS</p> <p>DT</p> <p>DU</p> <p>DV</p> <p>DW</p> <p>DX</p> <p>DY</p> <p>DZ</p> <p>EA</p> <p>EB</p> <p>EC</p> <p>ED</p> <p>EE</p> <p>EF</p> <p>EG</p> <p>EH</p> <p>EI</p> <p>EJ</p> <p>EK</p> <p>EL</p> <p>EM</p> <p>EN</p> <p>EO</p> <p>EP</p> <p>EQ</p> <p>ER</p> <p>ES</p> <p>ET</p> <p>EU</p> <p>EV</p> <p>EW</p> <p>EX</p> <p>EY</p> <p>EZ</p> <p>FA</p> <p>FB</p> <p>FC</p> <p>FD</p> <p>FE</p> <p>FF</p> <p>FG</p> <p>FH</p> <p>FI</p> <p>FJ</p> <p>FK</p> <p>FL</p> <p>FM</p> <p>FN</p> <p>FO</p> <p>FP</p> <p>FQ</p> <p>FR</p> <p>FS</p> <p>FT</p> <p>FU</p> <p>FV</p> <p>FW</p> <p>FX</p> <p>FY</p> <p>FZ</p> <p>GA</p> <p>GB</p> <p>GC</p> <p>GD</p> <p>GE</p> <p>GF</p> <p>GG</p> <p>GH</p> <p>GI</p> <p>GJ</p> <p>GK</p> <p>GL</p> <p>GM</p> <p>GN</p> <p>GO</p> <p>GP</p> <p>GQ</p> 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<p>KN</p> <p>KO</p> <p>KP</p> <p>KQ</p> <p>KR</p> <p>KS</p> <p>KT</p> <p>KU</p> <p>KV</p> <p>KW</p> <p>KX</p> <p>KY</p> <p>KZ</p> <p>LA</p> <p>LB</p> <p>LC</p> <p>LD</p> <p>LE</p> <p>LF</p> <p>LG</p> <p>LH</p> <p>LI</p> <p>LJ</p> <p>LK</p> <p>LL</p> <p>LM</p> <p>LN</p> <p>LO</p> <p>LP</p> <p>LQ</p> <p>LR</p> <p>LS</p> <p>LT</p> <p>LU</p> <p>LV</p> <p>LW</p> <p>LX</p> <p>LY</p> <p>LZ</p> <p>MA</p> <p>MB</p> <p>MC</p> <p>MD</p> <p>ME</p> <p>MF</p> <p>MG</p> <p>MH</p> <p>MI</p> <p>MJ</p> <p>MK</p> <p>ML</p> <p>MM</p> <p>MN</p> <p>MO</p> <p>MP</p> <p>MQ</p> <p>MR</p> <p>MS</p> <p>MT</p> <p>MU</p> <p>MV</p> <p>MW</p> <p>MX</p> <p>MY</p> <p>MZ</p> <p>NA</p> <p>NB</p> <p>NC</p> <p>ND</p> <p>NE</p> <p>NF</p> <p>NG</p> <p>NH</p> <p>NI</p> <p>NJ</p> <p>NK</p> <p>NL</p> <p>NM</p> <p>NN</p> <p>NO</p> <p>NP</p> <p>NQ</p> <p>NR</p> <p>NS</p> <p>NT</p> <p>NU</p> <p>NV</p> <p>NW</p> <p>NX</p> <p>NY</p> <p>NZ</p> <p>OA</p> <p>OB</p> <p>OC</p> <p>OD</p> <p>OE</p> <p>OF</p> <p>OG</p> <p>OH</p> <p>OI</p> <p>OJ</p> <p>OK</p> <p>OL</p> <p>OM</p> <p>ON</p> <p>OO</p> <p>OP</p> <p>OQ</p> <p>OR</p> <p>OS</p> <p>OT</p> <p>OU</p> <p>OV</p> <p>OW</p> <p>OX</p> <p>OY</p> <p>OZ</p> <p>PA</p> <p>PB</p> <p>PC</p> <p>PD</p> <p>PE</p> <p>PF</p> <p>PG</p> <p>PH</p> <p>PI</p> <p>PJ</p> <p>PK</p> <p>PL</p> <p>PM</p> <p>PN</p> <p>PO</p> <p>PP</p> <p>PQ</p> <p>PR</p> <p>PS</p> <p>PT</p> <p>PU</p> <p>PV</p> <p>PW</p> <p>PX</p> <p>PY</p> <p>PZ</p> <p>QA</p> <p>QB</p> <p>QC</p> <p>QD</p> <p>QE</p> <p>QF</p> <p>QG</p> <p>QH</p> <p>QI</p> <p>QJ</p> <p>QK</p> <p>QL</p> <p>QM</p> <p>QN</p> <p>QO</p> <p>QP</p> <p>QQ</p> <p>QR</p> <p>QS</p> <p>QT</p> <p>QU</p> <p>QV</p> <p>QW</p> <p>QX</p> <p>QY</p> <p>QZ</p> <p>RA</p> <p>RB</p> <p>RC</p> <p>RD</p> <p>RE</p> <p>RF</p> <p>RG</p> <p>RH</p> <p>RI</p> <p>RJ</p> <p>RK</p> <p>RL</p> <p>RM</p> <p>RN</p> <p>RO</p> <p>RP</p> <p>RQ</p> <p>RR</p> <p>RS</p> <p>RT</p> <p>RU</p> <p>RV</p> <p>RW</p> <p>RX</p> <p>RY</p> <p>RZ</p> <p>SA</p> <p>SB</p> <p>SC</p> <p>SD</p> <p>SE</p> <p>SF</p> <p>SG</p> <p>SH</p> <p>SI</p> <p>SJ</p> <p>SK</p> <p>SL</p> <p>SM</p> <p>SN</p> <p>SO</p> <p>SP</p> <p>SQ</p> <p>SR</p> <p>SS</p> <p>ST</p> <p>SU</p> <p>SV</p> <p>SW</p> <p>SX</p> <p>SY</p> <p>SZ</p> <p>TA</p> <p>TB</p> <p>TC</p> <p>TD</p> <p>TE</p> <p>TF</p> <p>TG</p> <p>TH</p> <p>TI</p> <p>TJ</p> <p>TK</p> <p>TL</p> <p>TM</p> <p>TN</p> <p>TO</p> <p>TP</p> <p>TQ</p> <p>TR</p> <p>TS</p> <p>TT</p> <p>TU</p> <p>TV</p> <p>TW</p> <p>TX</p> <p>TY</p> <p>TZ</p> <p>UA</p> <p>UB</p> <p>UC</p> <p>UD</p> <p>UE</p> <p>UF</p> <p>UG</p> <p>UH</p> <p>UI</p> <p>UJ</p> <p>UK</p> <p>UL</p> <p>UM</p> <p>UN</p> <p>UO</p> <p>UP</p> <p>UQ</p> <p>UR</p> <p>US</p> <p>UT</p> <p>UU</p> <p>UV</p> <p>UW</p> <p>UX</p> <p>UY</p> <p>UZ</p> <p>VA</p> <p>VB</p> <p>VC</p> <p>VD</p> <p>VE</p> <p>VF</p> <p>VG</p> <p>VH</p> <p>VI</p> <p>VJ</p> <p>VK</p> <p>VL</p> <p>VM</p> <p>VN</p> <p>VO</p> <p>VP</p> <p>VQ</p> <p>VR</p> <p>VS</p> <p>VT</p> <p>VU</p> <p>VV</p> <p>VW</p> <p>VX</p> <p>VY</p> <p>VZ</p> <p>WA</p> 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 Fax: 01608 665675
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 northam@mk-surveys.co.uk

www.surveys4bm.co.uk

GALLAGHER DEVELOPMENTS

RAF Cardington
 Bedfordshire

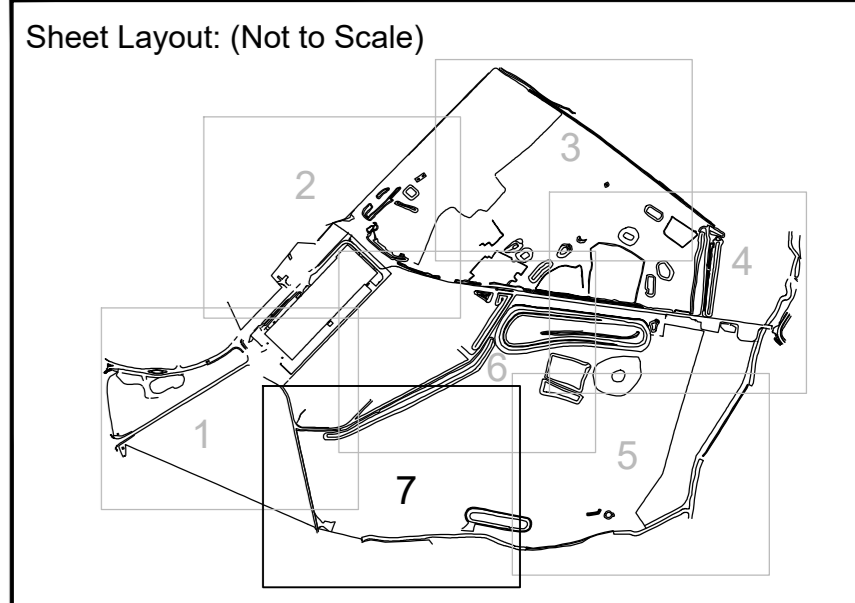
Topographical Survey

Scale: 1:500	Sheet Size: A0	Sheet Number: 6	Date: April 2024
Project Number: 28446	Rev: -	Surveyed by: SGB	Checked by: NS

- Notes:**
- GRID AND LEVELS BASED ON ORDNANCE DATUM, DERIVED FROM THE NATIONAL GRID NETWORK. LOCAL SCALE FACTOR 0.99974 APPLIED.
 - DRAINAGE INFORMATION HAS BEEN DETERMINED WITHOUT MAN ENTRY INTO CHAMBERS AND WHILST EVERY EFFORT HAS BEEN MADE TO CORRECTLY IDENTIFY THIS INFORMATION, IT SHOULD ALWAYS BE CHECKED IN AREAS THAT ARE CRITICAL TO THE FUTURE PROPOSAL.
 - ALL SEWERS ARE PRESUMED TO BE STRAIGHT BETWEEN CHAMBERS, WITH ROUTES CONNECTIVITY OBTAINED USING ACOUSTIC METHODS ONLY. THESE ARE TO BE CONSIDERED ASSUMED AND SHOULD BE INVESTIGATED FURTHER IN CRITICAL AREAS.
 - TREE AND HEDGE SPECIES HAVE BEEN IDENTIFIED AS ACCURATELY AS POSSIBLE BUT SHOULD BE CHECKED IN CRITICAL AREAS.

Coordinate Table				
Station	Description	Easting	Northing	Level
MKS100	ROAD NAIL	507678.670	246729.123	33.791
MKS101	ROAD NAIL	507771.680	246713.243	32.120
MKS102	ROAD NAIL	507658.363	246732.998	31.918
MKS103	ROAD NAIL	507928.398	246774.010	32.000
MKS104	ROAD NAIL	508019.581	246880.118	31.453
MKS105	ROAD NAIL	508120.678	246985.017	30.588

TOPOGRAPHICAL KEY	
	GENERAL ABBREVIATIONS
	ACU
	AV
	BD
	BL
	BO
	BS
	CA
	CB
	CC
	CD
	CE
	CF
	CG
	CH
	CI
	CJ
	CK
	CL
	CM
	CN
	CO
	CP
	CQ
	CR
	CS
	CT
	CU
	CV
	CW
	CX
	CY
	CZ
	DA
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	EN
	EO
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	EY
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	FY
	FZ
	GA
	GB
	GC
	GD
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	GI
	GJ
	GK
	GL
	GM
	GN
	GO
	GP



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 mk-surveys@mk-surveys.co.uk

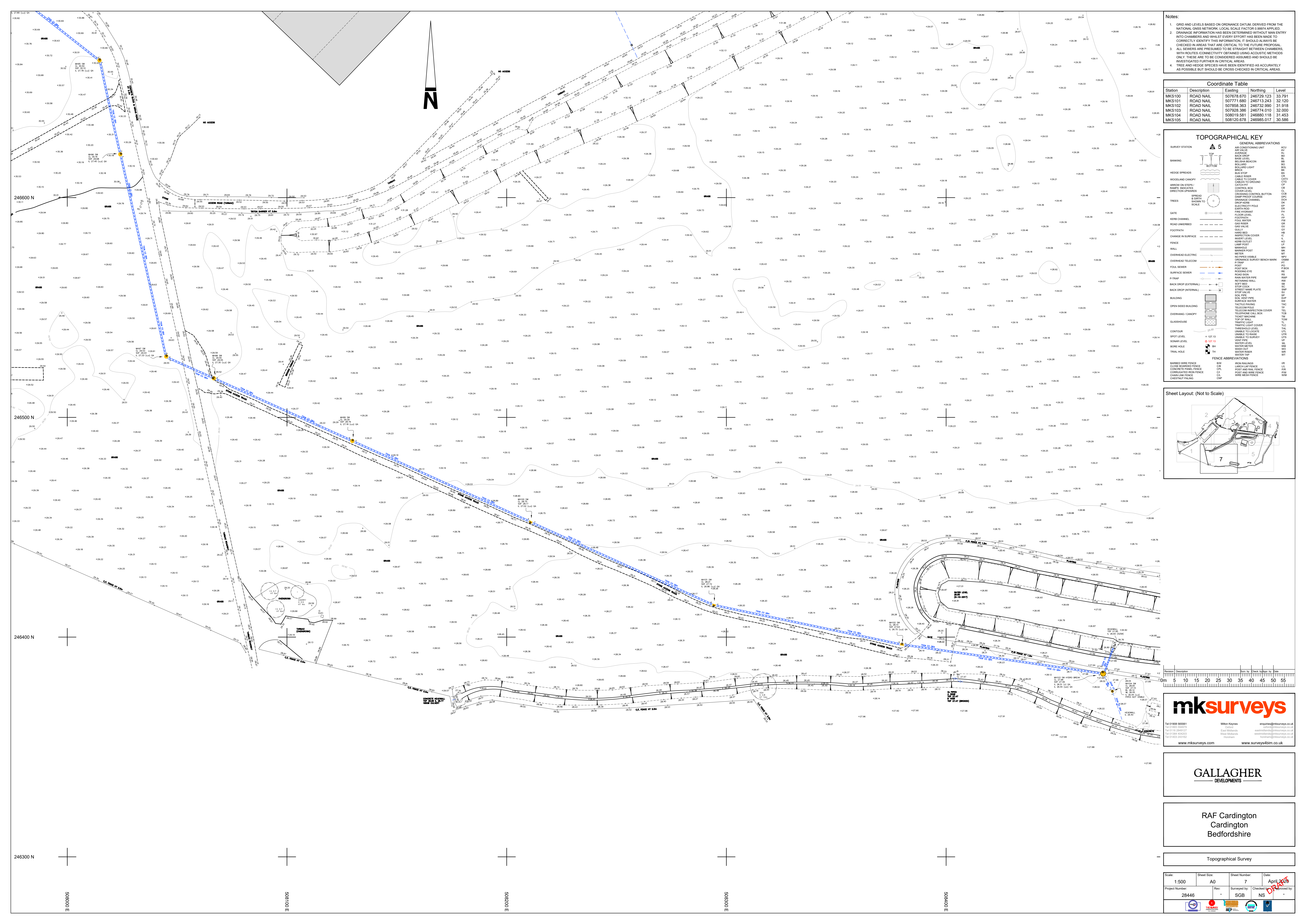
www.surveys4bm.co.uk

GALLAGHER
DEVELOPMENTS

RAF Cardington
Cardington
Bedfordshire

Topographical Survey

Scale: 1:500	Sheet Size: A0	Sheet Number: 7	Date: April 2024
Project Number: 28446	Rev: -	Surveyed by: SGB	Checked by: NS



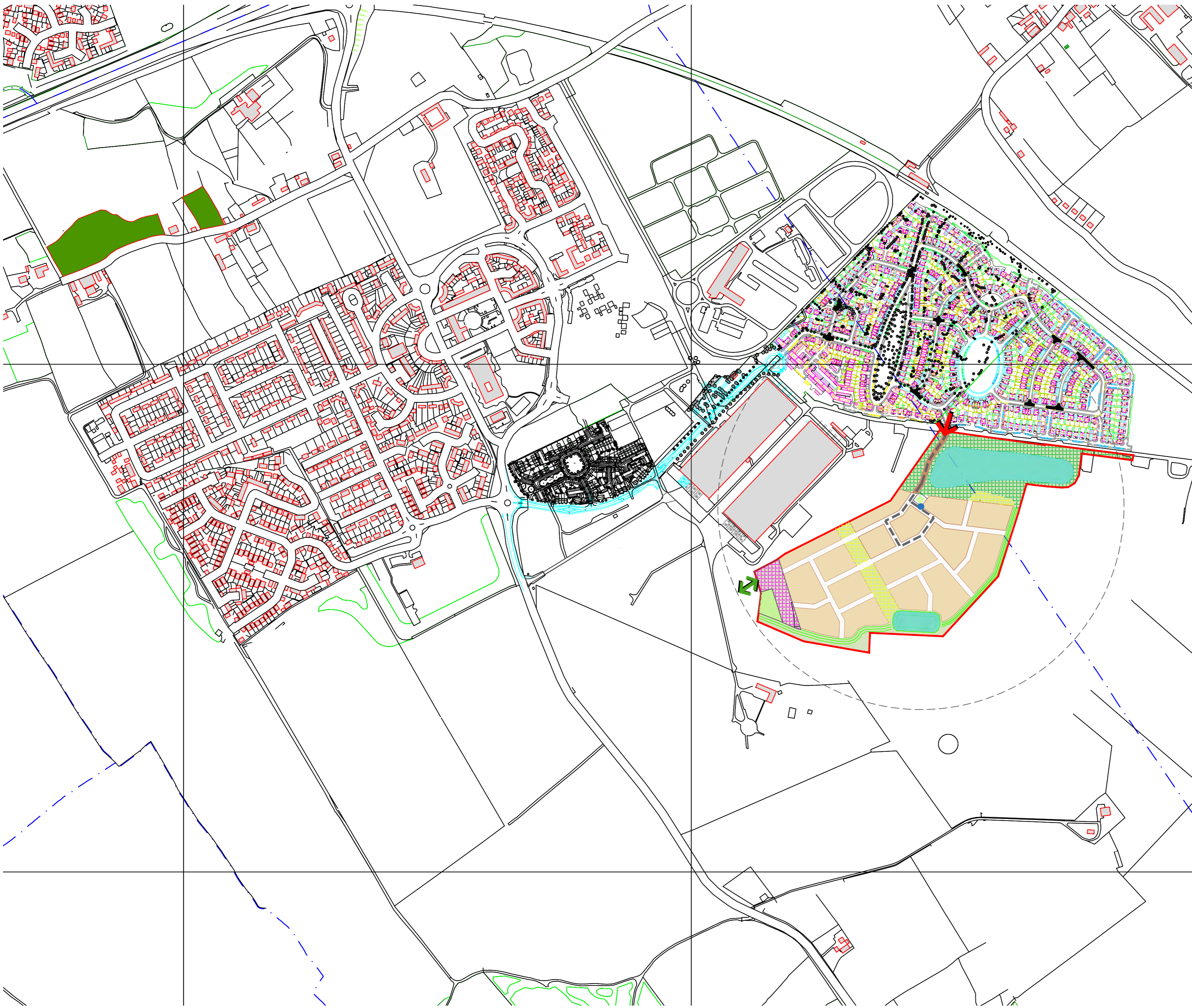


B. Development Proposals

Appendices



Cardington Retained Land
Project Number: WIE15662

Document Reference: WIE15662-109-BN-5-4-1-Flood

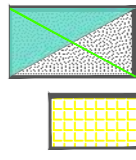












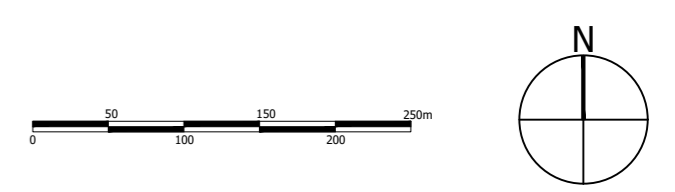
The scaling of this drawing cannot be assured

Revision	Date	Drn	Ckd
H	10.07.2020	DR	MV

-  Site Boundary 14.49 Ha
-  Residential Development 08.11 Ha

Public Open Space to meet LPA Policy AD28
(Of the total 06.21Ha POS Provision, 04.34Ha of this needs to meet the Forest of Marston Vale Policy)

-  Informal and Amenity Green Space 03.79 Ha
Including Basins, Swales and 0.71 Ha to contribute towards the Forest of Marston Vale Policy
-  Accessible Natural Green Space 01.73 Ha
-  Parks and Gardens 00.50 Ha
-  Equipped / Natural Play Areas 00.19 Ha
-  Main Infrastructure 00.17 Ha
-  Potential bus route
-  Potential bus stop
-  400m isochrone from bus stop
-  Potential Vehicular Access
-  Potential Pedestrian/Cycle Access to A600
-  Additional Land required to meet the Forest of MV Policy 01.90 Ha



Project
Land South of Cardington Hangars

Drawing Title
Concept Plan

Date	Scale	Drawn by	Check by
06.07.2020	1:5000 @A2	KU	MV
Project No	Drawing No	Revision	
31550	BL-M-03	H	

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Heritage • Graphic Communication •
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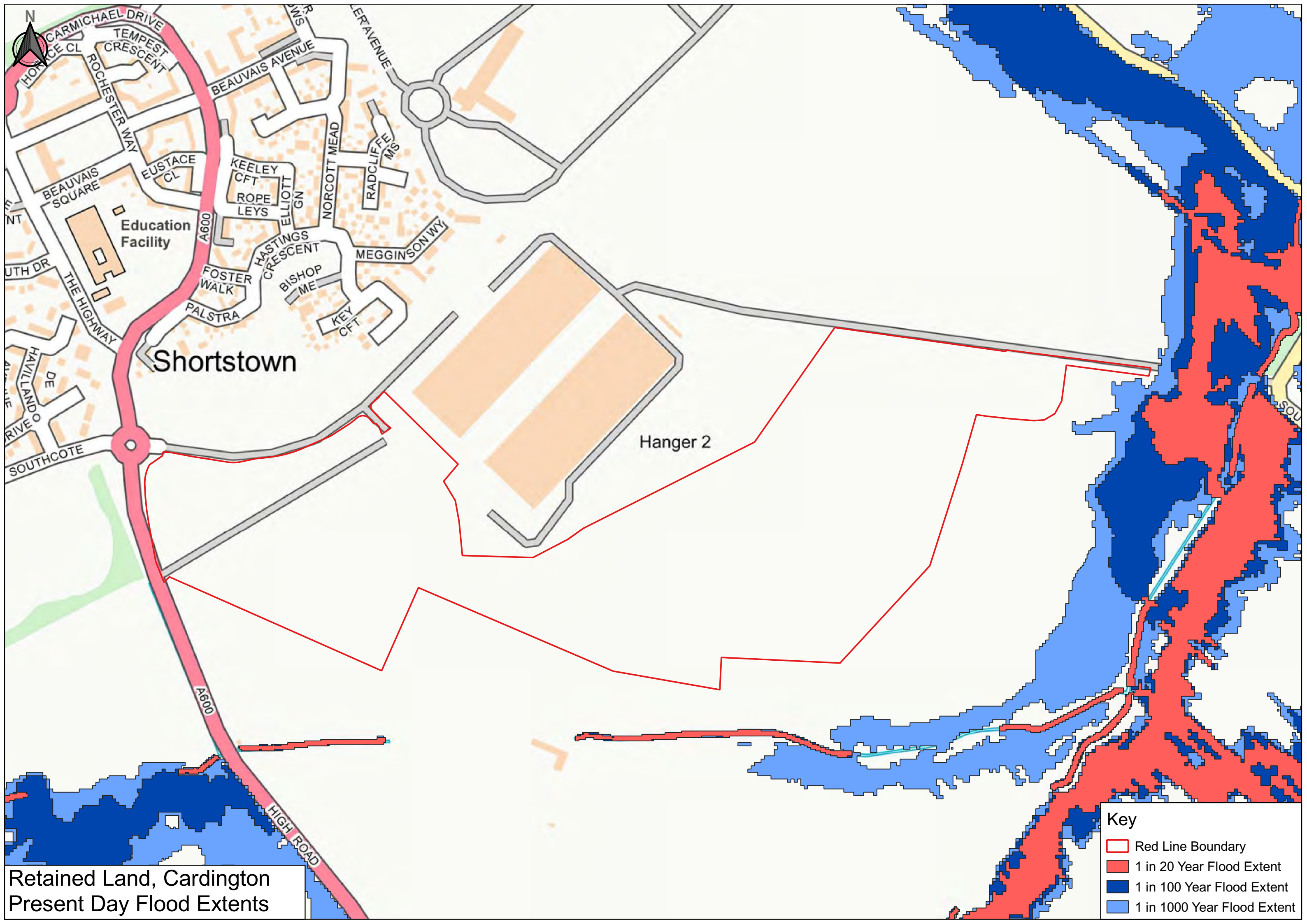


C. Hydraulic Modelling Results

Appendices





Cardington Retained Land
Project Number: WIE15662

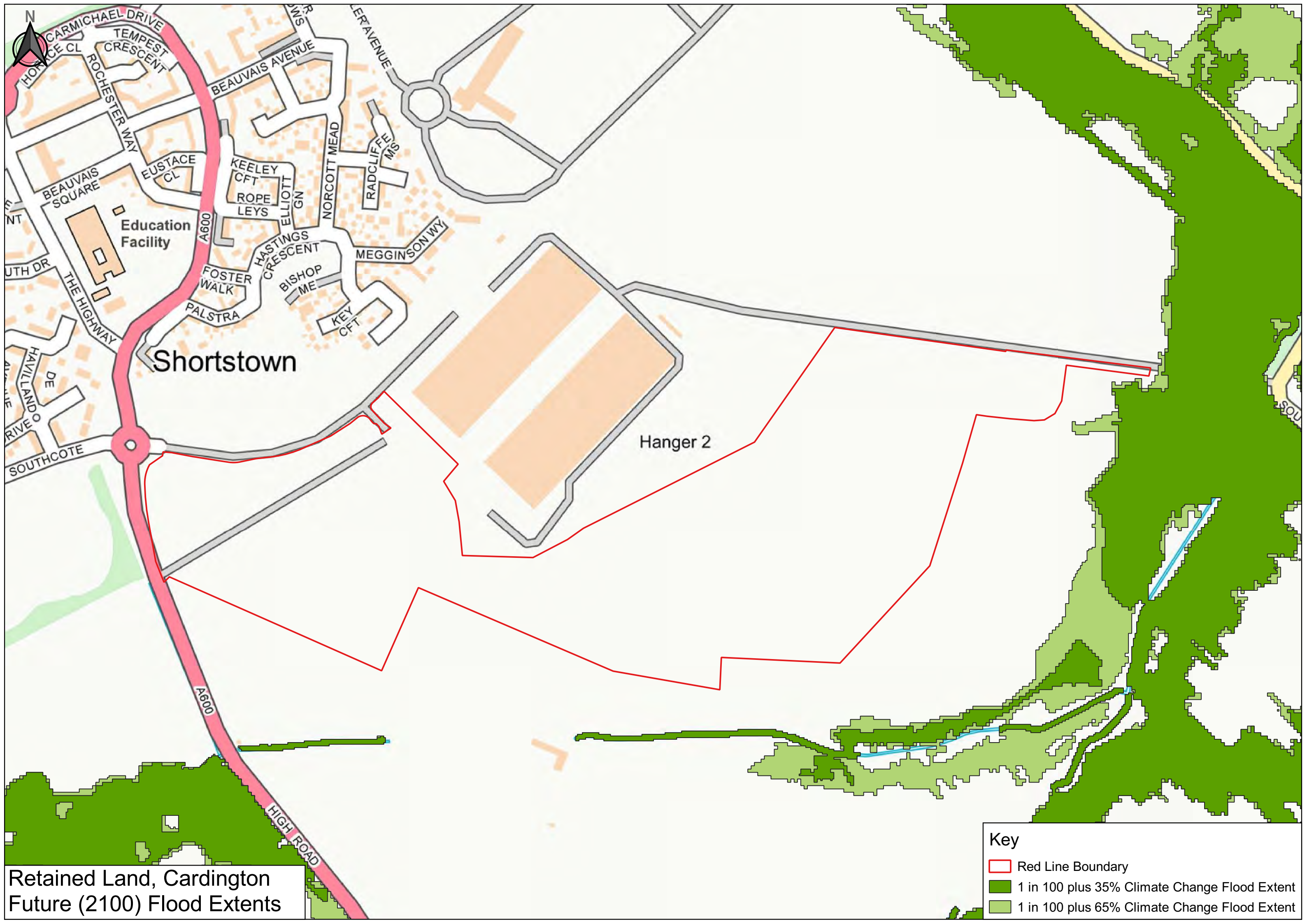
Document Reference: WIE15662-109-BN-5-4-1-Flood



Retained Land, Cardington
Present Day Flood Extents

Key

-  Red Line Boundary
-  1 in 20 Year Flood Extent
-  1 in 100 Year Flood Extent
-  1 in 1000 Year Flood Extent



Shortstown

Hanger 2

Key

- Red Line Boundary
- 1 in 100 plus 35% Climate Change Flood Extent
- 1 in 100 plus 65% Climate Change Flood Extent

Retained Land, Cardington
Future (2100) Flood Extents

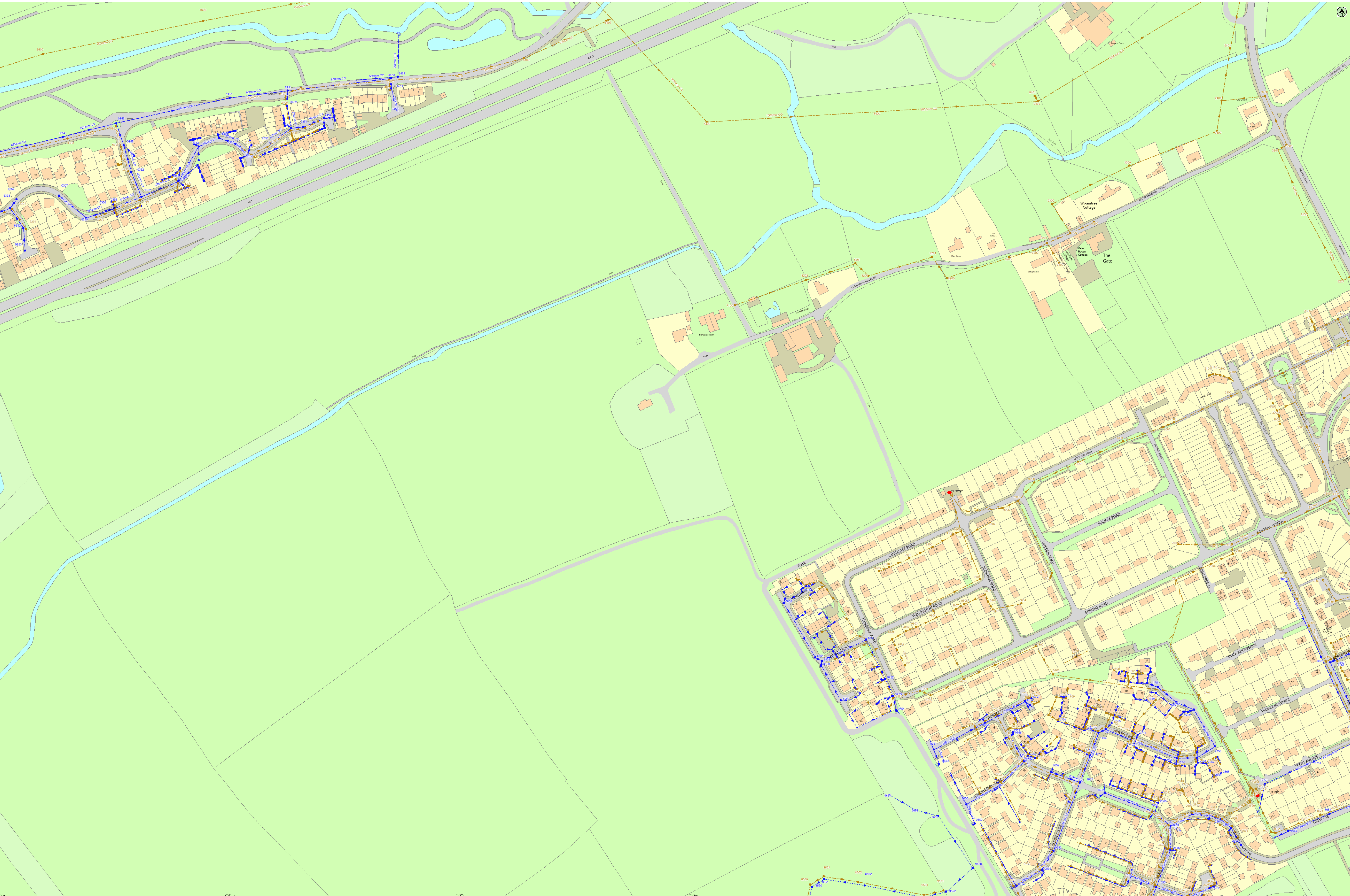


D. Anglian Water Sewer Records

Appendices

Cardington Retained Land
Project Number: WIE15662

Document Reference: WIE15662-109-BN-5-4-1-Flood



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Date: 14/05/19

Scale: 1:1250

Map Centre: 50679.247043

Data updated: 30/04/19

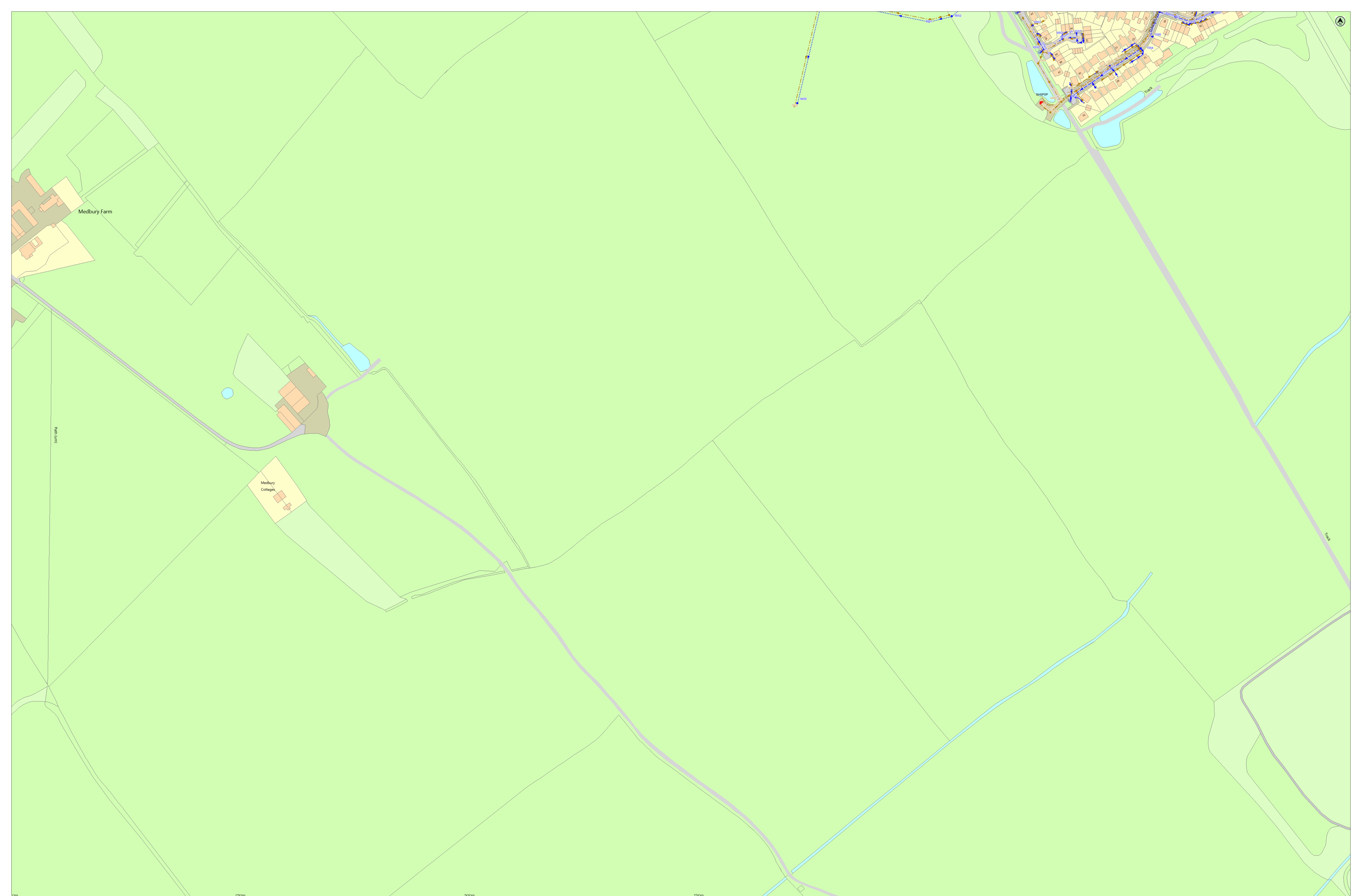
Our Ref: 311242-1

Wastewater Plan A0

Foul Sewer		Outfall		Sewage Treatment Works		Area 1		foran.odonovan@watermangroup.com
Surface Sewer		Inte*		Public Pumping Station		Area 1		
Combined Sewer		Manhole*		Decommissioned Pumping Station		Area 1		
Final Effluent								
Rising Main								
Private Sewer								
Decommissioned Sewer								







Manhole Reference	Easting	Northing	Liquid Type	Cover Level	Invert Level	Depth to Invert
0400	507090	246473	F	31.33	29.07	2.26
0401	507079	246462	F	31.17	28.96	2.21
0502	507071	246528	F	31.56	29.51	2.05
0503	507093	246541	F	31.72	29.74	1.98
0504	507065	246515	F	31.54	29.41	2.13
1400	507107	246485	F	31.24	28.75	2.49
1501	507198	246544	F	31.88	28.99	2.89
1502	507175	246531	F	31.57	28.93	2.64
1503	507110	246543	F	31.74	29.95	1.79
2500	507232	246562	F	31.87	29.84	2.03
7400	506800	246469	F	-	-	-
9500	506947	246563	F	-	-	-
9501	506969	246568	F	-	-	-
0550	507072	246530	S	31.59	30.34	1.25
0552	507092	246542	S	31.75	30.51	1.24
0553	507057	246555	S	31.88	30.53	1.35
1451	507102	246479	S	31.31	29.93	1.38
1553	507198	246571	S	31.94	30.27	1.67
1554	507177	246531	S	31.56	30.06	1.5
1555	507167	246545	S	31.62	30.09	1.53
1557	507108	246544	S	31.76	30.85	1.11
2550	507232	246561	S	31.82	30.59	1.23
2551	507256	246571	S	32	30.89	1.11
8450	506803	246472	S	-	-	-
9551	506947	246560	S	-	-	-
9552	506971	246567	S	-	-	-

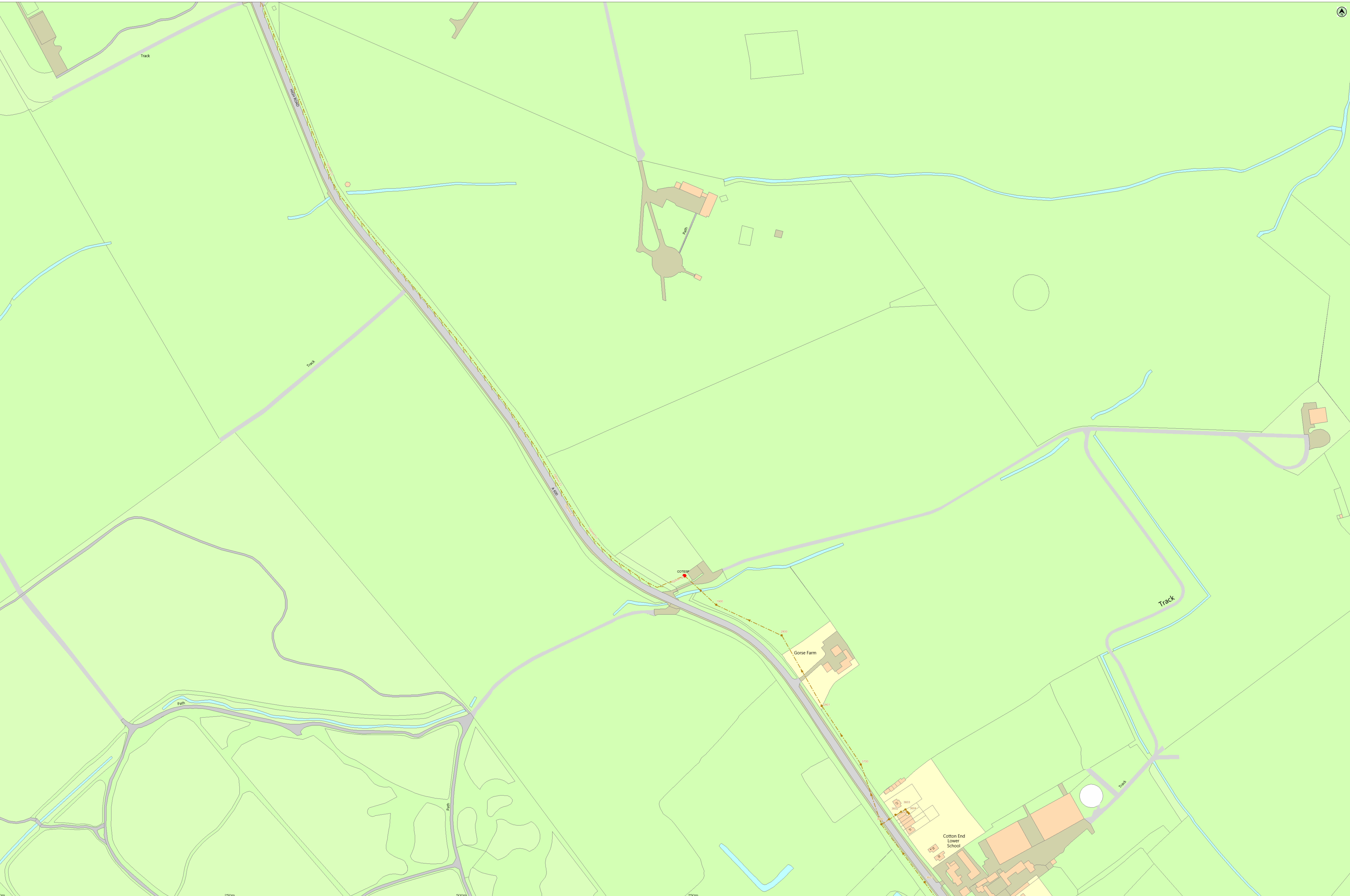
Manhole Reference	Easting	Northing	Liquid Type	Cover Level	Invert Level	Depth to Invert
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Manhole Reference	Easting	Northing	Liquid Type	Cover Level	Invert Level	Depth to Invert
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Manhole Reference	Easting	Northing	Liquid Type	Cover Level	Invert Level	Depth to Invert
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Manhole Reference	Easting	Northing	Liquid Type	Cover Level	Invert Level	Depth to Invert
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Manhole Reference	Easting	Northing	Liquid Type	Cover Level	Invert Level	Depth to Invert
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Date: 14/05/19

Scale: 1:1250

Map Centre: 508122,246081

Data updated: 30/04/19

Our Ref: 311242 - 4

Wastewater Plan A0

Foul Sewer		Outfall	
Surface Sewer		Inlet	
Combined Sewer		Manhole	
Final Effluent		Decommissioned Sewer	
Rising Main			
Private Sewer			
Decommissioned Sewer			

	Sewage Treatment Works		Area 4
	Public Pumping Station		
	Decommissioned Pumping Station		

stonar.odonovan@watermangroup.com

Area 4



Manhole Reference	Easting	Northing	Liquid Type	Cover Level	Invert Level	Depth to Invert
1900	508166	245915	F	-	-	-
2800	508237	245882	F	-	-	-
2801	508280	245906	F	-	-	-
3600	508344	245678	F	-	-	-
3601	508391	245615	F	-	-	-
3602	508360	245688	F	-	-	-
3603	508371	245694	F	-	-	-
3604	508374	245689	F	-	-	-
3700	508322	245742	F	-	-	-

Manhole Reference	Easting	Northing	Liquid Type	Cover Level	Invert Level	Depth to Invert
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Manhole Reference	Easting	Northing	Liquid Type	Cover Level	Invert Level	Depth to Invert
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Manhole Reference	Easting	Northing	Liquid Type	Cover Level	Invert Level	Depth to Invert
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Manhole Reference	Easting	Northing	Liquid Type	Cover Level	Invert Level	Depth to Invert
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Manhole Reference	Easting	Northing	Liquid Type	Cover Level	Invert Level	Depth to Invert
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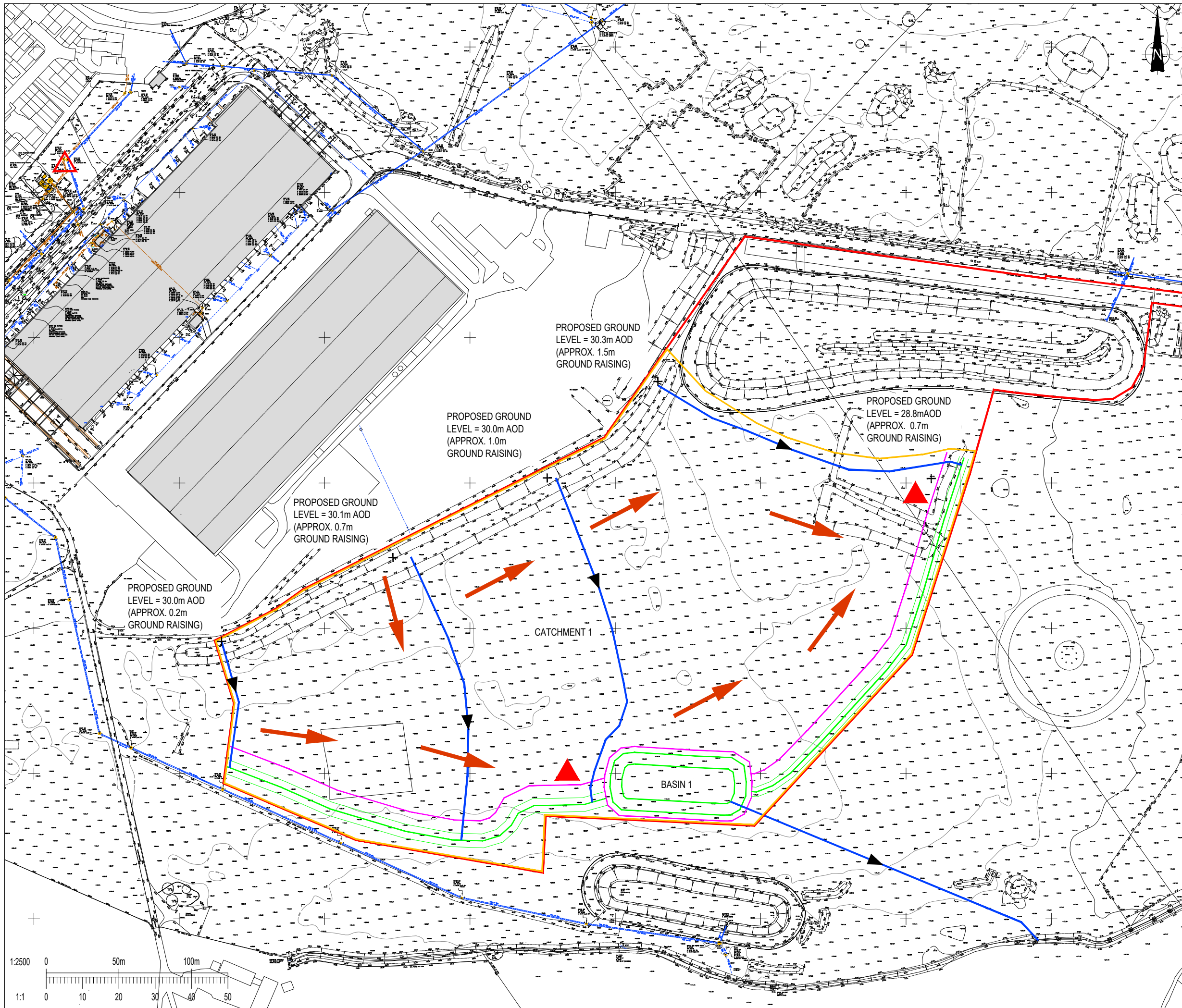


E. Preliminary Drainage Strategy

Appendices

Cardington Retained Land
Project Number: WIE15662

Document Reference: WIE15662-109-BN-5-4-1-Flood



KEY

- SITE BOUNDARY
- CATCHMENT BOUNDARY
- PROPOSED SURFACE WATER CONNECTION
- PROPOSED SWALE
- ▭ PROPOSED DETENTION BASIN (DRY)
- ▭ MAINTENANCE BUFFER
- ➔ PROPOSED GRAVITY FOUL DRAINAGE ROUTES
- ▲ PROPOSED FOUL PUMPING STATION
- ▲/ EXISTING FOUL PUMPING STATION

Rev	Date	Description	By
P04	27.07.20	RED LINE BOUNDARY UPDATED	CH
P03	05.05.20	RED LINE BOUNDARY UPDATED	CH
P02	24.04.20	DRAINAGE STRATEGY AMENDED	CH
P01	08.04.20	PRELIMINARY	CH

Amendments

Project

CARDINGTON RETAINED LAND

Title

PRELIMINARY DRAINAGE STRATEGY

Client

Gallagher Developments Group Ltd

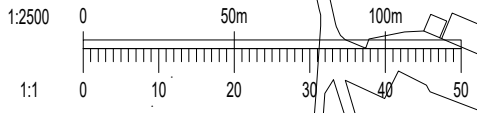


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 mail@watermangroup.com www.watermangroup.com

Status

PRELIMINARY

Designed By	CH	Checked By	DO	Waterman Ref	WIE15662
Drawn By	CH	Date	APRIL 2020	Scales @ A3	1:2500
Project - Originator - Volume - Level - Type - Role - Number					Revision
15662-WIE-ZZ-XX-DR-92001					P04





F. Surface Water Runoff Calculations

Appendices

Cardington Retained Land
Project Number: WIE15662

Document Reference: WIE15662-109-BN-5-4-1-Flood

CALCULATIONS

Company: WIE Office: London
 Sheet No: 1 of 3 Project No: WIE15662
 By: C Henderson Date: 28.07.20
 Checked: D O'Donovan Date: 28.07.20

Project Title: Cardington Retained Land
 Calculations Title: Surface Water Management - Summary Sheet

LOCATION	CALCULATIONS	OPTIONS										
	<p>Surface water at the Site will be managed in accordance with latest guidance and industry best practice, i.e. surface water discharge restricted to the greenfield runoff rate.</p>											
	<p>Existing surface water discharge regime:</p> <p>Greenfield land discharging into watercourses at the greenfield runoff rate</p>											
	<p>Proposed surface water discharge regime:</p> <p>Restrict to 4 l/s/ha of impermeable area for the 1 in 100 year event + 40% climate change as requested by the Bedfordshire and River Ivel Internal Drainage Board</p> <p>Discharge rate = 4.0 l/s/ha applied to the impermeable for each catchment</p>											
	<p>Initial attenuation estimates</p> <table border="1" data-bbox="242 1249 1289 1368"> <thead> <tr> <th>Catchment</th> <th>Area (ha)</th> <th>Effective Area (ha)*</th> <th>Discharge Rate (l/s)</th> <th>Attenuation (m³)**</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>8.11</td> <td>6.24</td> <td>25.0</td> <td>4955</td> </tr> </tbody> </table>	Catchment	Area (ha)	Effective Area (ha)*	Discharge Rate (l/s)	Attenuation (m ³)**	1	8.11	6.24	25.0	4955	
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	<p>*Includes 70% Percentage Impermeable Area (PIMP) and 10% urban creep allowance as per Bedford Borough Council requirement, total PIMP 77%.</p>											
	<p>**The total storage required for the entire site has been calculated then pro-rated down to the effective area for each catchment</p>											

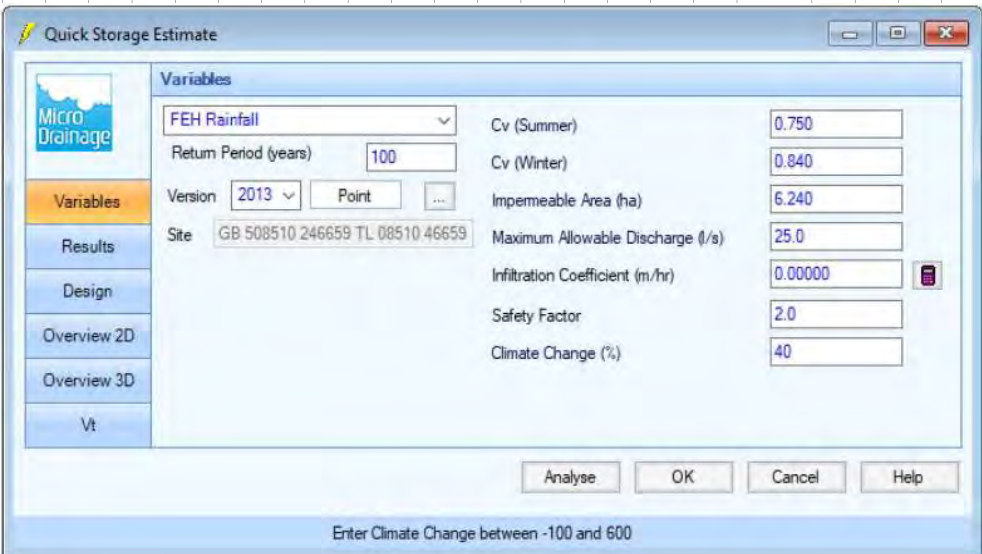
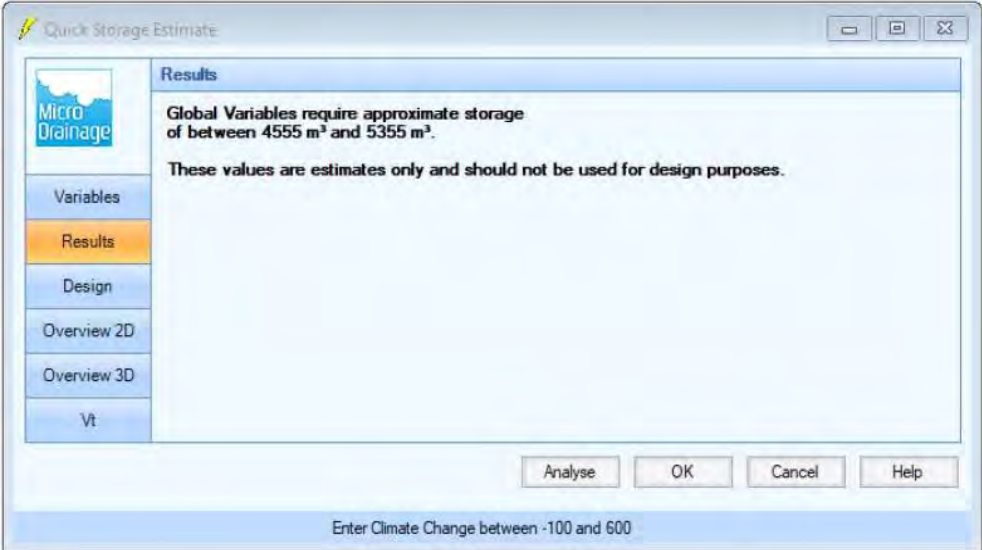
CALCULATIONS

Company: WIE
 Sheet No: 2 of 3
 By: C Henderson
 Checked: D O'Donovan

Office: London
 Project No: WIE15662
 Date: 28.07.20
 Date: 28.07.20

Project Title: Cardington Retained Land




Calculations Title: Preliminary Surface Water Attenuation Volume - Greenfield Rate

LOCATION	CALCULATIONS	OPTIONS										
	<p>In order to calculate the volume of surface water attenuation required for the Site, Windes Microdrainage version 2018.1, Source Control module, Quick Storage Estimate has been used. The input and output data for which are shown below;</p> <p>Input:</p> 											
	<p>Output:</p>  <p>As Windes Quick Storage Estimate provides a range of attenuation volumes it is considered that an average value of the range is suitable for preliminary design sizing.</p> <table border="1" data-bbox="236 1933 1222 2016"> <tr> <td>Minimum:</td> <td>4,555</td> <td>m³</td> <td></td> <td></td> </tr> <tr> <td>Maximum:</td> <td>5,355</td> <td>m³</td> <td>Preliminary Estimate:</td> <td>4955 m³</td> </tr> </table>	Minimum:	4,555	m ³			Maximum:	5,355	m ³	Preliminary Estimate:	4955 m ³	
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CALCULATIONS

Company: WIE Office: London
 Sheet No: 3 of 3 Project No: WIE15662
 By: C Henderson Date: 28.07.20
 Checked: D O'Donovan Date: 28.07.20

Project Title: Cardington Retained Land
 Calculations Title: Greenfield Runoff Rate (IoH)

LOCATION	CALCULATIONS	OPTIONS																																																								
	<p>In order to calculate the rate of surface water discharge from the permeable portion of the site, the Windes Microdrainage version 2018.1 Source Control module has been utilised. Rural runoff has been calculated using the IoH 124 Methodology. The input and output data for which are shown below;</p> <div data-bbox="280 813 1254 1675" style="border: 1px solid black; padding: 10px; margin: 10px auto; width: fit-content;"> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td colspan="2">Waterman Group</td> <td>Page 1</td> </tr> <tr> <td colspan="2">Pickfords Wharf Clink Street London, SE1 9DG</td> <td rowspan="2" style="text-align: center;"></td> </tr> <tr> <td>Date 02/04/2020 14:25</td> <td>Designed by CSCH3</td> </tr> <tr> <td>File</td> <td>Checked by</td> <td></td> </tr> <tr> <td>Innovyze</td> <td colspan="2">Source Control 2019.1</td> </tr> </table> <p style="text-align: center; margin-top: 10px;"><u>IH 124 Mean Annual Flood</u></p> <p style="text-align: center;">Input</p> <table style="margin-left: auto; margin-right: auto;"> <tr> <td>Return Period (years)</td> <td>100</td> <td>Soil</td> <td>0.400</td> </tr> <tr> <td>Area (ha)</td> <td>50.000</td> <td>Urban</td> <td>0.000</td> </tr> <tr> <td>SAAR (mm)</td> <td>547</td> <td>Region Number</td> <td>Region 5</td> </tr> </table> <p style="text-align: center; margin-top: 10px;">Results l/s</p> <table style="margin-left: auto; margin-right: auto;"> <tr> <td>QBAR Rural</td> <td>127.5</td> </tr> <tr> <td>QBAR Urban</td> <td>127.5</td> </tr> <tr> <td>Q100 years</td> <td>453.8</td> </tr> <tr> <td>Q1 year</td> <td>110.9</td> </tr> <tr> <td>Q2 years</td> <td>113.9</td> </tr> <tr> <td>Q5 years</td> <td>164.4</td> </tr> <tr> <td>Q10 years</td> <td>211.0</td> </tr> <tr> <td>Q20 years</td> <td>266.5</td> </tr> <tr> <td>Q25 years</td> <td>288.3</td> </tr> <tr> <td>Q30 years</td> <td>306.3</td> </tr> <tr> <td>Q50 years</td> <td>362.3</td> </tr> <tr> <td>Q100 years</td> <td>453.8</td> </tr> <tr> <td>Q200 years</td> <td>534.1</td> </tr> <tr> <td>Q250 years</td> <td>559.6</td> </tr> <tr> <td>Q1000 years</td> <td>734.3</td> </tr> </table> </div>	Waterman Group		Page 1	Pickfords Wharf Clink Street London, SE1 9DG			Date 02/04/2020 14:25	Designed by CSCH3	File	Checked by		Innovyze	Source Control 2019.1		Return Period (years)	100	Soil	0.400	Area (ha)	50.000	Urban	0.000	SAAR (mm)	547	Region Number	Region 5	QBAR Rural	127.5	QBAR Urban	127.5	Q100 years	453.8	Q1 year	110.9	Q2 years	113.9	Q5 years	164.4	Q10 years	211.0	Q20 years	266.5	Q25 years	288.3	Q30 years	306.3	Q50 years	362.3	Q100 years	453.8	Q200 years	534.1	Q250 years	559.6	Q1000 years	734.3	
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Attenuation Design

WIE15662 - Cardington Retained Land

28.07.20

STORAGE REQUIRED PER CATCHMENT	Catchment 1	4955 m³												
<table border="1"><tr><td>DETENTION BASIN 1</td><td></td></tr><tr><td>Area of base</td><td>1800.0 m²</td></tr><tr><td>Perimeter of base</td><td>190.0 m</td></tr><tr><td>Depth of storage</td><td>2.00 m</td></tr><tr><td>Gradient of sides</td><td>1 in 4</td></tr><tr><td>Volume</td><td>5120 m³</td></tr></table>			DETENTION BASIN 1		Area of base	1800.0 m ²	Perimeter of base	190.0 m	Depth of storage	2.00 m	Gradient of sides	1 in 4	Volume	5120 m³
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Volume	5120 m³													
TOTAL STORAGE PER CATCHMENT		5120 m³												

UK and Ireland Office Locations

